

Revised Minimum and Guidance Levels Based on Reevaluation of Levels Adopted for Pierce Lake in Pasco County, Florida



July 18, 2019

Resource Evaluation Section
Water Resources Bureau
Southwest Florida
Water Management District

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Contents

Definitions.....	2
Introduction.....	7
Reevaluation of Minimum Flows and Levels	7
Minimum Flows and Levels Program Overview.....	7
Legal Directives	7
Development of Minimum Lake Levels in the Southwest Florida Water Management District.....	9
Programmatic Description and Major Assumptions.....	9
Consideration of Changes and Structural Alterations and Environmental Values	10
Lake Classification	13
Minimum and Guidance Levels.....	14
Development of Minimum and Guidance Levels for Pierce Lake	15
Lake Setting and Description	15
Land Use Land Cover.....	17
Bathymetry Description and History	23
Water Level (Lake Stage) Record	23
Historic Management Levels	25
Methods, Results and Discussion.....	25
Bathymetry	26
Development of Exceedance Percentiles	28
Normal Pool Elevation and Additional Information	29
Guidance Levels	29
Significant Change Standards	30
Minimum Levels.....	32
Consideration of Environmental Values	34
Comparison of Revised and Previously Adopted Levels	35
Minimum Levels Status Assessment	36
Documents Cited and Reviewed.....	37
Appendix A: Pierce Lake Water Budget Model, Rainfall Correlation Model, and Historic Percentile Estimations	
Appendix B: Pierce Lake Initial Minimum Levels Status Assessment	
Appendix C: Evaluation of Groundwater Withdrawal Impacts to Pierce Lake	

Definitions

<i>Category 1 Lakes</i>	Lakes with lake-fringing cypress swamp(s) greater than 0.5 acre in size where Structural Alterations have not prevented the Historic P50 from equaling or rising above an elevation that is 1.8 feet below the Normal Pool elevation of the cypress swamp(s).
<i>Category 2 Lakes</i>	Lakes with lake-fringing cypress swamp(s) greater than 0.5 acre in size where Structural Alterations have prevented the Historic P50 from equaling or rising above an elevation that is 1.8 feet below the Normal Pool and the lake fringing cypress swamp(s) remain viable and perform functions beneficial to the lake despite the Structural Alterations.
<i>Category 3 Lakes</i>	Lakes without lake-fringing cypress swamp(s) greater than 0.5 acre in size.
<i>Control Point Elevation</i>	The elevation of the highest stable point along the outlet profile of a surface water conveyance system that principally controls lake water level fluctuations
<i>Current</i>	A recent Long-term period during which Structural Alterations and hydrologic stresses are stable.
<i>District</i>	Southwest Florida Water Management District (SWFWMD)
<i>Dynamic Ratio</i>	The ratio of a lake's surface area (in square kilometers) to the mean depth of the lake (in meters). Used to determine at what water level a lake is susceptible to decreased water quality, i.e., turbidity, due to wave disturbance of bottom sediments.
<i>F.A.C.</i>	Florida Administrative Code

<i>FDEP</i>	Florida Department of Environmental Protection
<i>F.S.</i>	Florida Statutes
<i>Guidance Levels</i>	Water levels determined by the District and used as advisory information for the District, lake shore residents and local governments, or to aid in the management or control of adjustable structures.
<i>High Guidance Level (HGL)</i>	The expected Historic P10 elevation. Provided as an advisory guideline for the construction of lake shore development, water dependent structures, and operation of water management structures.
<i>High Minimum Lake Level (HMLL)</i>	The elevation that a lake's water levels are required to equal or exceed ten percent of the time on a Long-term basis
<i>Historic</i>	A Long-term period when there are no measurable impacts due to withdrawals, and Structural Alterations are similar to current conditions.
<i>Historic P10</i>	The expected Historic P10 elevation; <i>i.e.</i> , the elevation of the water surface of a lake or wetland that is expected to be equaled or exceeded ten percent of the time based on a Long-term period when there are or were no measurable impacts due to withdrawals, and Structural Alterations are similar to current conditions.
<i>Historic P50</i>	The expected Historic P50 elevation; <i>i.e.</i> , the elevation of the water surface of a lake or wetland that is expected to be equaled or exceeded fifty percent of the time based on a Long-term period when there are or were no measurable impacts due to withdrawals, and Structural Alterations are similar to current conditions.

Historic P90

The expected Historic P90 elevation; /i.e., the elevation of the water surface of a lake or wetland that is expected to be equaled or exceeded ninety percent of the time based on a Long-term period when there are or were no measurable impacts due to withdrawals, and Structural Alterations are similar to current conditions.

Hydrologic Indicators

Biological and physical features, as listed In Section 373.4211 (20), Florida Statutes, which are representative or indicative of previous water levels.

Leakance

Relative to groundwater movement, the ratio of the vertical hydrologic conductivity of the confining bed to the thickness of the confining bed (Anderson and Woessner, 2002); a measure of how easily water can pass through a confining unit.

Long-term

An evaluation period utilized to establish minimum flows and levels, to determine compliance with established minimum flows and levels, and to assess withdrawal impacts on established minimum flows and levels, that represents a period which spans the range of hydrologic conditions which can be expected to occur based upon historical records, ranging from high water levels to low water levels. In the context of a predictive model simulation, a Long-term simulation will be insensitive to temporal fluctuations in withdrawal rates and hydrologic conditions, so as to simulate steady-state, average conditions. In the context of an average water level, the average will be based upon the historic expected range and frequency of levels. relative to minimum level establishment and compliance, where there are six years or more of competent data, a minimum of a six-year evaluation period will be used; but the available data and reasonable scientific judgement will dictate whether a longer period is used. Where there are less than six years of competent data, the period used will be dictated by the available data and a determination, based on reasonable scientific

judgement, that the period is sufficiently representative of Long-term conditions.

*Low Guidance Level
(LGL)*

The expected Historic P90. Provided as an advisory guideline for construction of water dependent structures, information for lakeshore residents, and operation of water management structures.

MFL

Minimum Flows and Levels

*Minimum Lake Level
(MLL)*

The elevation that the lake's water levels are required to equal or exceed fifty percent of the time on a Long-term basis.

NAVD 88

North American Vertical Datum of 1988

NGVD 29

National Geodetic Vertical Datum of 1929

Normal Pool Elevation

An elevation approximating the P10 (see below) elevation which is determined based on hydrologic indicators of sustained inundation

Not Structurally Altered

Refers to a lake where the control point elevation equals or exceeds the Normal Pool elevation, or the lake has no outlet

P10

The percentile ranking represented by the elevation of the water surface of a lake or wetland that is equaled or exceeded ten percent of the time as determined from a Long-term stage frequency analysis.

P50

The percentile ranking represented by the elevation of the water surface of a lake or wetland that is equaled or exceeded fifty percent of the time as determined from a Long-term stage frequency analysis.

<i>P90</i>	The percentile ranking represented by the elevation of the water surface of a lake or wetland that is equaled or exceeded ninety percent of the time as determined from a Long-term stage frequency analysis.
<i>Reference Lakes</i>	Lakes from a defined area which are not measurably impacted by water withdrawals. Reference lakes may be used to develop reference lake statistics, including the RLWR50, RLWR90, and the RLWR5090 (see below).
<i>RLWR50</i>	Reference Lake Water Regime 50. The median difference between the P10 and P50 elevations for reference lakes with historic data and similar hydrogeologic conditions as the lake of concern.
<i>RLWR5090</i>	Reference Lake Water Regime 5090. The median difference between the P50 and P90 elevations for reference lakes with historic data and similar hydrogeologic conditions as the lake of concern.
<i>RLWR90</i>	Reference Lake Water Regime 90. The median difference between the P10 and P90 lake stage elevations for reference lakes with historic data and similar hydrogeologic conditions as the lake of concern
<i>SFWMD</i>	South Florida Water Management District
<i>SJRWMD</i>	St. Johns River Water Management District
<i>SWFWMD</i>	Southwest Florida Water Management District

Introduction

Reevaluation of Minimum Flows and Levels

This report describes the development of minimum levels and guidance levels for Pierce Lake in Pasco County, Florida. These levels were developed based on the reevaluation of minimum and guidance levels approved by the Southwest Florida Water Management District (District) Governing Board in November 2006 and subsequently adopted into District rules. The minimum and guidance levels represent necessary revisions to the currently adopted levels.

Pierce Lake was selected for reevaluation based on development of modeling tools used to simulate natural water level fluctuations in lake basins that were not available when the currently adopted minimum levels for the lake were developed. Adopted levels for Pierce Lake were also reevaluated to support ongoing District assessment of minimum flows and levels and the need for additional recovery in the Northern Tampa Bay Water Use Caution Area (NTB WUCA), a region of the District where recovery strategies are being implemented to support recovery to minimum flow and level thresholds.

Minimum Flows and Levels Program Overview

Legal Directives

Section 373.042, Florida Statutes (F.S.), directs the Department of Environmental Protection or the water management districts to establish minimum flows and levels (MFLs) for lakes, wetlands, rivers and aquifers. Section 373.042(1)(a), F.S., states that "[t]he minimum flow for a given watercourse shall be the limit at which further withdrawals would be significantly harmful to the water resources or ecology of the area." Section 373.042(1)(b), F.S., defines the minimum water level of an aquifer or surface water body as "...the level of groundwater in an aquifer and the level of surface water at which further withdrawals would be significantly harmful to the water resources of the area." MFLs are established and used by the Southwest Florida Water Management District (SWFWMD or District) for water resource planning, as one of the criteria used for evaluating water use permit applications, and for the design, construction and use of surface water management systems.

Established MFLs are key components of resource protection, recovery and regulatory compliance, as Section 373.0421(2) F.S., requires the development of a recovery or prevention strategy for water bodies "[i]f the existing flow or level in a water body is below, or is projected to fall within 20 years below, the applicable minimum flow or level established pursuant to S. 373.042." Section 373.0421(2)(a), F.S., requires that recovery or prevention strategies be developed to: "(a) [a]chieve recovery to the established minimum flow or level as soon as practicable; or (b) [p]revent the existing flow or level from falling below the established minimum flow or level." Periodic reevaluation and, as necessary, revision of established minimum flows and levels are required by Section 373.0421(3), F.S.

Minimum flows and levels are to be established based upon the best information available, and when appropriate, may be calculated to reflect seasonal variations (Section 373.042(1), F.S.). Also, establishment of MFLs is to involve consideration of, and at the governing board or department's discretion, may provide for the protection of nonconsumptive uses (Section 373.042(1), F.S.). Consideration must also be given to "...changes and structural alterations to watersheds, surface waters and aquifers, and the effects such changes or alterations have had, and the constraints such changes or alterations have placed, on the hydrology of the affected watershed, surface water, or aquifer...", with the requirement that these considerations shall not allow significant harm caused by withdrawals (Section 373.0421(1)(a), F.S.). Sections 373.042 and 373.0421 provide additional information regarding the prioritization and scheduling of minimum flows and levels, the independent scientific review of scientific or technical data, methodologies, models and scientific and technical assumptions employed in each model used to establish a minimum flow or level, and exclusions that may be considered when identifying the need for MFLs establishment.

The Florida Water Resource Implementation Rule, specifically Rule 62-40.473, Florida Administrative Code (F.A.C.), provides additional guidance for the establishment of MFLs, requiring that "...consideration shall be given to natural seasonal fluctuations in water flows or levels, nonconsumptive uses, and environmental values associated with coastal, estuarine, riverine, spring, aquatic and wetlands ecology, including: a) Recreation in and on the water; b) Fish and wildlife habitats and the passage of fish; c) estuarine resources; d) Transfer of detrital material; e) Maintenance of freshwater storage and supply; f) Aesthetic and scenic attributes; g) Filtration and absorption of nutrients and other pollutants; h) Sediment loads; i) Water quality; and j) Navigation."

Rule 62-40.473, F.A.C., also indicates that "[m]inimum flows and levels should be expressed as multiple flows or levels defining a minimum hydrologic regime, to the extent practical and necessary to establish the limit beyond which further withdrawals would be significantly harmful to the water resources or the ecology of the area as provided in Section 373.042(1), F.S." It further notes that, "...a minimum flow or level need not be expressed as multiple flows or levels if other resource protection tools, such as reservations implemented to protect fish and wildlife or public health and safety, that provide equivalent or greater protection of the hydrologic regime of the water body, are developed and adopted in coordination with the minimum flow or level." The rule also includes provision addressing: protection of MFLs during the construction and operation of water resource projects; the issuance of permits pursuant to Section 373.086 and Parts II and IV of Chapter 373, F.S.; water shortage declarations; development of recovery or prevention strategies, development and updates to a minimum flow and level priority list and schedule, and peer review for MFLs establishment.

Development of Minimum Lake Levels in the Southwest Florida Water Management District

Programmatic Description and Major Assumptions

Since the enactment of the Florida Water Resources Act of 1972 (Chapter 373, F.S.), in which the legislative directive to establish MFLs originated, and following subsequent modifications to this directive and adoption of relevant requirements in the Water Resource Implementation Rule, the District has actively pursued the adoption, i.e., establishment of MFLs for priority water bodies. The District implements established MFLs primarily through its water supply planning, water use permitting and environmental resource permitting programs, and through the funding of water resource and water supply development projects that are part of a recovery or prevention strategy. The District's MFLs program addresses all relevant requirements expressed in the Florida Water Resources Act and the Water Resource Implementation Rule.

A substantial portion of the District's organizational resources has been dedicated to its MFLs Program, which logistically addresses six major tasks: 1) development and reassessment of methods for establishing MFLs; 2) adoption of MFLs for priority water bodies (including the prioritization of water bodies and facilitation of public and independent scientific review of proposed MFLs and methods used for their development); 3) monitoring and MFLs status assessments, i.e., compliance evaluations; 4) development and implementation of recovery strategies; 5) MFLs compliance reporting; and 6) ongoing support for minimum flow and level regulatory concerns and prevention strategies. Many of these tasks are discussed or addressed in this Minimum Levels report; additional information on all tasks associated with the District's MFLs Program is summarized by Hancock *et al.* (2010).

The District's MFLs Program is implemented based on three fundamental assumptions. First, it is assumed that many water resource values and associated features are dependent upon and affected by long-term hydrology and/or changes in long-term hydrology. Second, it is assumed that relationships between some of these variables can be quantified and used to develop significant harm thresholds or criteria that are useful for establishing MFLs. Third, the approach assumes that alternative hydrologic regimes may exist that differ from non-withdrawal impacted conditions but are sufficient to protect water resources and the ecology of these resources from significant harm.

Support for these assumptions is provided by a large body of published scientific work addressing relationships between hydrology, ecology and human-use values associated with water resources (e.g., see reviews and syntheses by Postel and Richter 2003, Wantzen *et al.* 2008, Poff *et al.* 2010, Poff and Zimmerman 2010). This information has been used by the District and other water management districts within the state to identify significant harm thresholds or criteria supporting development of MFLs for hundreds of water bodies, as summarized in the numerous publications associated with these efforts (e.g., SFWMD 2000, 2006, Flannery *et al.* 2002, SRWMD 2004, 2005, Neubauer *et al.* 2008, Mace 2009).

With regard to the assumption associated with alternative hydrologic regimes, consider a historic condition for an unaltered river or lake system with no local groundwater or surface water withdrawal impacts. A new hydrologic regime for the system would be associated with each increase in water use, from small withdrawals that have no measurable effect on the historic regime to large withdrawals that could substantially alter the regime. A threshold hydrologic regime may exist that is lower or less than the historic regime, but which protects the water resources and ecology of the system from significant harm. This threshold regime could conceptually allow for water withdrawals, while protecting the water resources and ecology of the area. Thus, MFLs may represent minimum acceptable rather than historic or potentially optimal hydrologic conditions.

Consideration of Changes and Structural Alterations and Environmental Values

When establishing MFLs, the District considers "...changes and structural alterations to watersheds, surface waters and aquifers, and the effects such changes or alterations have had, and the constraints such changes or alterations have placed, on the hydrology of the affected watershed, surface water, or aquifer..." in accordance with Section 373.0421(1)(a), F.S. Also, as required by statute, the District does not establish MFLs that would allow significant harm caused by withdrawals when considering the changes, alterations and their associated effects and constraints. These considerations are based on review and analysis of best available information, such as water level records, environmental and construction permit information, water control structure and drainage alteration histories, and observation of current site conditions.

When establishing, reviewing or implementing MFLs, considerations of changes and structural alterations may be used to:

- adjust measured flow or water level historical records to account for existing changes/alterations;
- model or simulate flow or water level records that reflect long-term conditions that would be expected based on existing changes/alterations and in the absence of measurable withdrawal impacts;
- develop or identify significant harm standards, thresholds and other criteria;
- aid in the characterization or classification of lake types or classes based on the changes/alterations;
- evaluate the status of water bodies with proposed or established MFLs (i.e., determine whether the flow and/or water level are below, or are projected to fall below the applicable minimum flow or level); and
- support development of lake guidance levels (described in the following paragraph).

The District has developed specific methodologies for establishing minimum flows or levels for lakes, wetlands, rivers, estuaries and aquifers, subjected the methodologies to independent, scientific peer-review, and incorporated the methods for some system types, including lakes, into its Water Level and Rates of Flow rules (Chapter 40D-8, F.A.C.). The rules also provide for the establishment of Guidance Levels for lakes,

which serve as advisory information for the District, lakeshore residents and local governments, or to aid in the management or control of adjustable water level structures.

Information regarding the development of adopted methods for establishing minimum and guidance lake levels is included in Southwest Florida Water Management District (1999a, b) and Leeper *et al.* (2001). Additional information relevant to developing lake levels is presented by Schultz *et al.* (2004), Carr and Rochow (2004), Caffrey *et al.* (2006, 2007), Carr *et al.* (2006), Hancock (2006), Hoyer *et al.* (2006), Leeper (2006), Hancock (2006, 2007) and Emery *et al.* (2009). Independent scientific peer-review findings regarding the lake level methods are summarized by Bedient *et al.* (1999), Dierberg and Wagner (2001) and Wagner and Dierberg (2006).

For lakes, methods have been developed for establishing Minimum Levels for systems with fringing cypress-dominated wetlands greater than 0.5 acre in size, and for those without fringing cypress wetlands. Lakes with fringing cypress wetlands where water levels currently rise to an elevation expected to fully maintain the integrity of the wetlands are classified as Category 1 Lakes. Lakes with fringing cypress wetlands that have been structurally altered such that lake water levels do not rise to levels expected to fully maintain the integrity of the wetlands are classified as Category 2 Lakes. Lakes with less than 0.5 acre of fringing cypress wetlands are classified as Category 3 Lakes.

Categorical significant change standards and other available information are developed to identify criteria that are sensitive to long-term changes in hydrology and can be used for establishing minimum levels. For all lake categories, the most sensitive, appropriate criterion or criteria are used to develop minimum levels. For Category 1 or 2 Lakes, a significant change standard, referred to as the Cypress Standard, is developed. The Cypress Standard is 1.8 feet below the normal pool elevation. For Category 3 lakes, six significant change standards are typically developed. Other available information, including potential changes in the coverage of herbaceous wetland and submersed aquatic plants, is also considered when establishing minimum levels for Category 3 Lakes. The standards and other available information are associated with the environmental values identified for consideration in Rule 62-40.473, F.A.C., when establishing MFLs (Table 1). The specific standards and other information evaluated to support development of minimum levels for Pierce Lake are provided in subsequent sections of this report. More general information on the standards and other information used for consideration when developing minimum lake levels is available in the documents identified in the preceding sub-section of this report.

Table 1: Environmental values from the Water Resource Implementation Rule (62-40.473, F.A.C.), and the Significant Change Standards (and other information) associated with each that are considered when establishing minimum flows and levels.

Environmental Value	Associated Significant Change Standards and Other Information for Consideration
Recreation in and on the water	Basin Connectivity Standard, Recreation/Ski Standard, Aesthetics Standard, Species Richness Standard, Dock-Use Standard, Herbaceous Wetland Information, Submersed Aquatic Macrophyte Information
Fish and wildlife habitats and the passage of fish	Cypress Standard, Wetland Offset, Basin Connectivity Standard, Species Richness Standard, Herbaceous Wetland Information, Submersed Aquatic Macrophyte Information
Estuarine resources	NA ¹
Transfer of detrital material	Cypress Standard, Wetland Offset, Basin Connectivity Standard, Lake Mixing Standard, Herbaceous Wetland Information, Submersed Aquatic Macrophyte Information
Maintenance of freshwater storage and supply	NA ²
Aesthetic and scenic attributes	Cypress Standard, Dock-Use Standard, Wetland Offset, Aesthetics Standard, Species Richness Standard, Herbaceous Wetland Information, Submersed Aquatic Macrophyte Information
Filtration and absorption of nutrients and other pollutants	Cypress Standard Wetland Offset Lake Mixing Standard Herbaceous Wetland Information Submersed Aquatic Macrophyte Information
Sediment loads	NA ¹
Water quality	Cypress Standard, Wetland Offset, Lake Mixing Standard, Dock-Use Standard, Herbaceous Wetland Information, Submersed Aquatic Macrophyte Information
Navigation	Basin Connectivity Standard, Submersed Aquatic Macrophyte Information

NA¹ = Not applicable for consideration for most priority lakes;

NA² = Environmental value is addressed generally by development of minimum levels based on appropriate significant change standards and other information and use of minimum levels in District permitting programs

Lake Classification

Lakes are classified as Category 1, 2, or 3 for Minimum Levels development. According to Rule 40D-8.624, F.A.C., Pierce Lake meets the classification as a Category 3 lake, with less than 0.5 acre of fringing cypress wetlands. The standards associated with Category 3 lakes described below will also be developed in a subsequent section of this report.

Lake-specific significant change standards and other available information are developed for establishing Minimum Levels for Category 3 Lakes. The standards are used to identify thresholds for preventing significant harm to cultural and natural system values associated with lakes in accordance with guidance provided in the Florida Water Resource Implementation Rule (62-40.473, F.A.C.). Other information taken into consideration includes potential changes in the coverage of herbaceous wetland vegetation and aquatic plants.

The Recreation/Ski Standard is developed to identify the lowest elevation within the lake basin that will contain an area suitable for safe water skiing. The standard is based on the lowest elevation within the basin that can contain a 5-foot deep ski corridor delineated as a circular area with a radius of 418 feet, or a rectangular ski corridor 200 feet in width and 2,000 feet in length (the Ski Elevation) and use of Historic lake stage data or region-specific Reference Lake Water Regime statistics where Historic lake data are not available.

The Dock-Use Standard is developed to provide for sufficient water depth at the end of existing docks to permit mooring of boats and prevent adverse impacts to bottom-dwelling plants and animals caused by boat operation. The standard is based on the elevation of lake sediments at the end of existing docks, a two-foot water depth for boat mooring, and use of Historic lake stage data or region-specific Reference Lake Water Regime statistics.

The Wetland Offset Elevation is developed to protect lake fringing non-cypress wetlands. Based on the rationale used to develop the Cypress Wetland Standard for Category 1 and 2 lakes (1.8 feet below the Normal Pool elevation), a Wetland Offset Elevation for Category 3 Lakes was developed. Because Hydrologic Indicators of sustained inundation used to determine the Normal Pool elevation usually do not exist on Category 3 Lakes, another datum, in this case the Historic P50 elevation, was used in the development of the Wetland Offset Elevation. Based on an evaluation of the relationship of the Cypress Wetland Standard with the Historic P50 for hydrologically unimpacted cypress wetlands, the Wetland Offset Elevation for Category 3 Lakes was established at an elevation 0.8 feet below the Historic P50 elevation (Hancock, draft report, 2007).

The Aesthetics Standard is developed to protect aesthetic values associated with the inundation of lake basins. The standard is intended to protect aesthetic values associated with the median lake stage from diminishing beyond the values associated

with the lake when it is staged at the Low Guidance Level. The Aesthetics Standard is established at the Low Guidance Level.

The Species Richness Standard is developed to prevent a decline in the number of bird species that may be expected to occur at or utilize a lake. Based on an empirical relationship between lake surface area and the number of birds expected to occur at a lake, the standard is established at the lowest elevation associated with less than a fifteen percent reduction in lake surface area relative to the lake area at the Historic P50 elevation.

The Basin Connectivity Standard is developed to protect surface water connections between lake basins or among sub-basins within lake basins to allow for movement of aquatic biota, such as fish, and support recreational use of the lake. The standard is based on the elevation of lake sediments at a critical high spot between lake basins or lake sub-basins, identification of water depths sufficient for movement of biota and/or watercraft across the critical high spot and use of Historic lake stage data or the region-specific Reference Lake Water Regime statistics where Historic lake data are not available.

The Lake Mixing Standard is developed to prevent significant changes in patterns of wind-driven mixing of the lake water column and sediment re-suspension. The standard is established at the highest elevation at or below the Historic P50 elevation where the dynamic ratio (see Bachmann *et al.* 2000) shifts from a value of <0.8 to a value >0.8 , or from a value >0.8 to a value of <0.8 .

Herbaceous Wetland Information is also taken into consideration to determine the elevation at which changes in lake stage would result in substantial changes in potential wetland area within the lake basin (i.e., basin area with a water depth of four feet or less) (Butts *et al.* 1997). Similarly, changes in lake stage associated with changes in lake area available for colonization by rooted submersed or floating-leaved macrophytes are also evaluated, based on water transparency values. Using methods described in Caffrey (2006), mean Secchi disk depth (SD) is used to calculate the maximum depth of colonization (MDC) for aquatic plants using regression equation $\log(\text{MDC}) = 0.66\log(\text{SD}) + 0.30$, where all values are represented in meters. The MDC depth is then used to calculate the total acreage at each lake stage that is available for aquatic plant colonization.

Minimum and Guidance Levels

Two Minimum Levels and two Guidance Levels are typically established for lakes. Upon completion of a public input/review process and, if necessary, completion of an independent scientific review, either of which may result in modification of the proposed levels, the levels are then adopted by the District Governing Board into Chapter 40D-8, F.A.C. (see Hancock *et al.* 2010 for more information on the adoption process). The levels, which are expressed as elevations in feet above the National Geodetic Vertical Datum of 1929 (NGVD29), include the following (refer to Rule 40D-8.624, F.A.C.):

- A **High Guidance Level** that is provided as an advisory guideline for construction of lake shore development, water dependent structures, and operation of water management structures. The High Guidance Level is the elevation that a lake's water levels are expected to equal or exceed ten percent of the time on a long-term basis.
- A **High Minimum Lake Level** that is the elevation that a lake's water levels are *required* to equal or exceed ten percent of the time on a long-term basis.
- A **Minimum Lake Level** that is the elevation that the lake's water levels are *required* to equal or exceed fifty percent of the time on a long-term basis.
- A **Low Guidance Level** that is provided as an advisory guideline for water dependent structures, information for lakeshore residents and operation of water management structures. The Low Guidance Level is the elevation that a lake's water levels are expected to equal or exceed ninety percent of the time on a long-term basis.

The District is in the process of converting from use of the NGVD29 datum to use of the North American Vertical Datum of 1988 (NAVD 88). While the NGVD29 datum is used for most elevation values included within this report, in some circumstances, notations are made for elevation data that was collected or reported relative to mean sea level or relative to NAVD88 and converted to elevations relative to NGVD29.

Development of Minimum and Guidance Levels for Pierce Lake

Lake Setting and Description

Pierce Lake (Figure 1) is located in Pasco County, Florida (Section 09, Township 25, Range 18) within the 171 square mile Pithlachascotee River/Bear Creek Watershed within the larger Coastal Rivers Basin within the Southwest Florida Water Management District.

Within the primary Coastal Rivers Basin, the lake's watershed (Figure 2) has a drainage area of approximately 115 acres including the lake. The lake has no direct inlet, and one main outlet into a small ditch on the southern lake edge (Figure 3). There are currently no surface water withdrawals from the lake permitted by the District. There are, however, several permitted groundwater withdrawals in the lake vicinity.

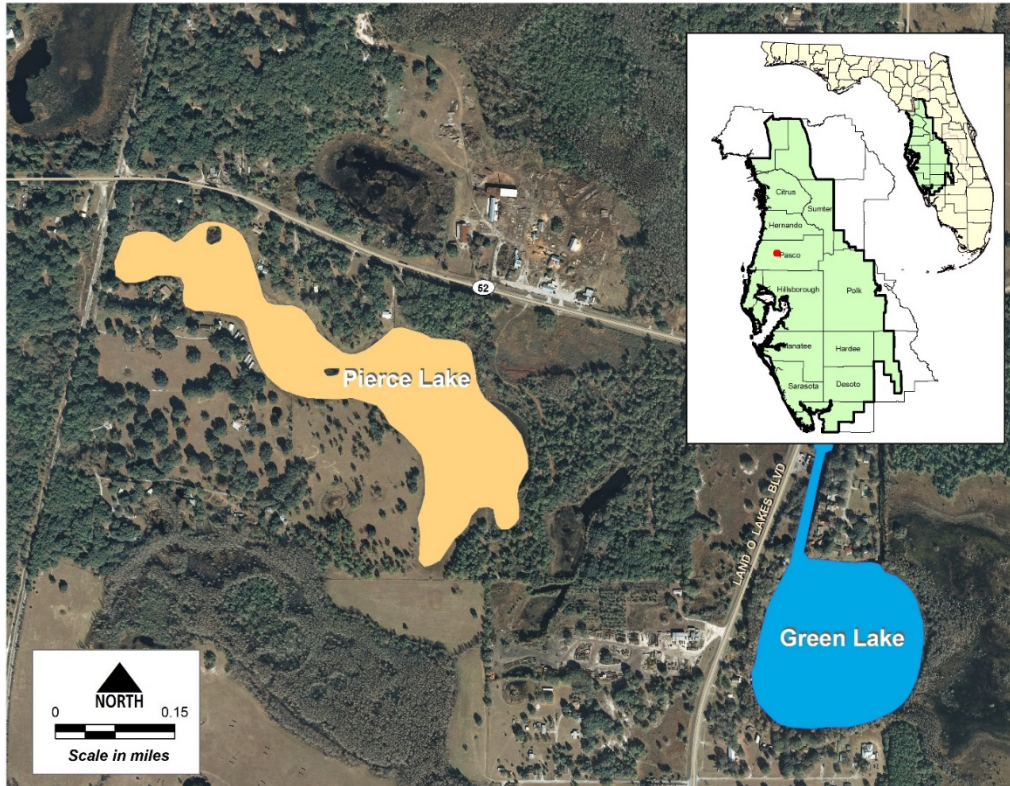


Figure 1: Location of Pierce Lake in Pasco County, Florida

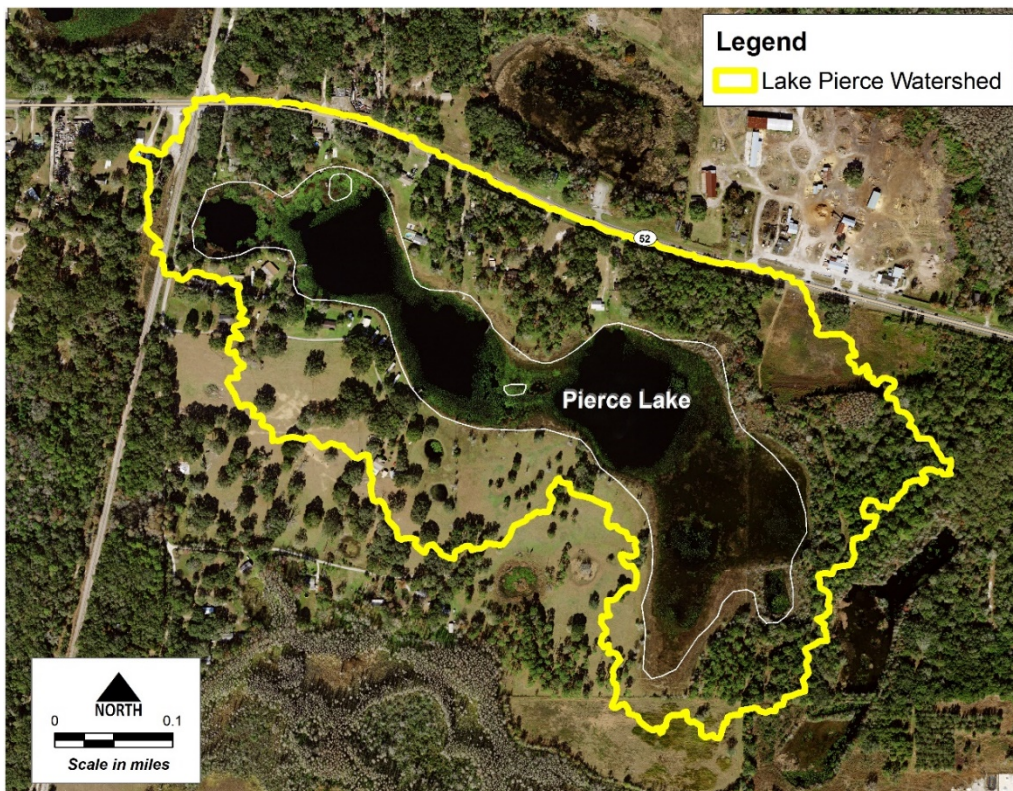


Figure 2: Watershed Delineation

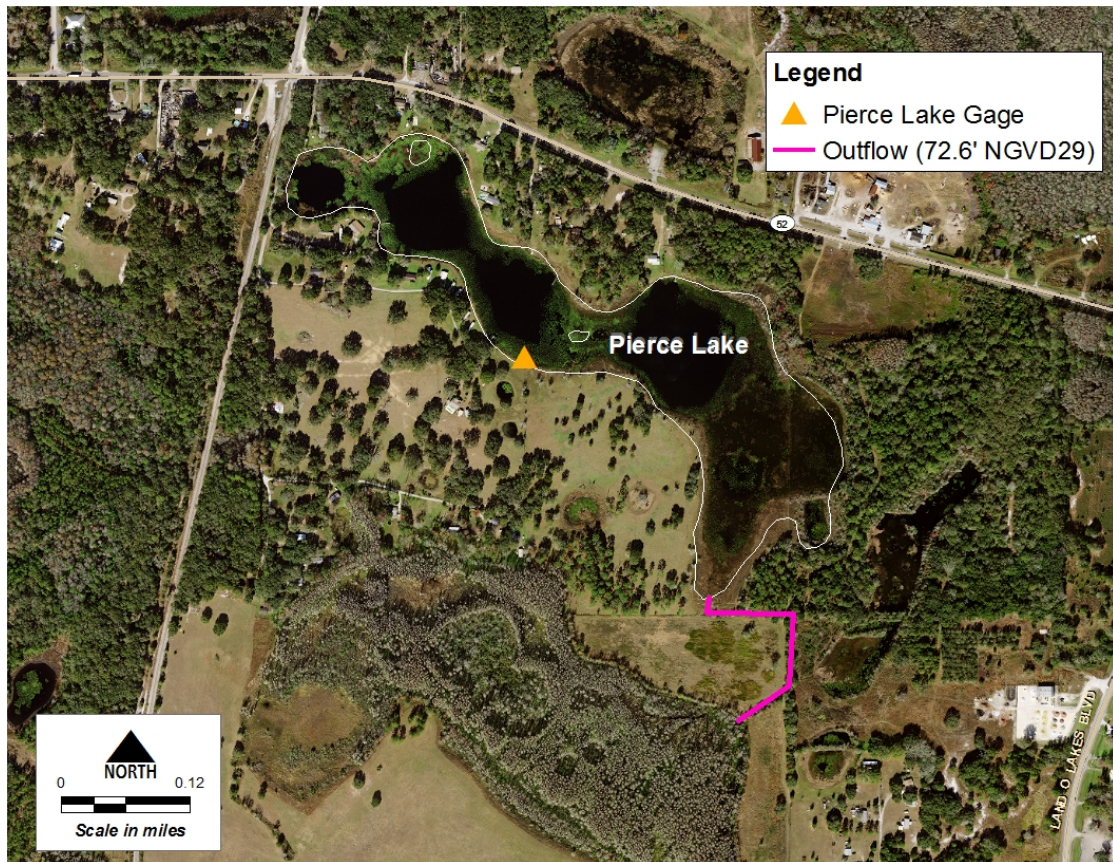


Figure 3: Location of Conveyance Systems and District Gages

Land Use Land Cover

An examination of the 1990, and most recent 2011 Florida Land Use, Cover, and Forms Classification System (FLUCCS) maps revealed that there has been little change to the landscape (specifically the dominant land forms) in the vicinity during this period (Figure 4 and Figure 5). In both years, the majority of the land surrounding Pierce Lake is classified as either residential, pine flatwoods, or pastureland. The sawmill occupying approximately 33 acres to the north side of SR52 was classified industrial and appears to have been part of the landscape since the mid-1970's. Unlike the lakes further south into Hillsborough County, Pierce Lake has seen little residential development around the lake. Most FLUCCS classification changes observed during this period have occurred within the wetland and lake ecosystems. Much of the wet areas classified as Emergent Aquatic Vegetation (e.g., Water Lily, Duckweed) in 1990 changed to Freshwater Marsh (e.g., Sawgrass, Cattail, Arrowhead, Buttonbush) and Wet Prairie (assorted wetland grasses) in 2011.

In Figure 6 through Figure 13, aerial photography chronicles landscape changes to the immediate lake basin from 1941 through 2012.

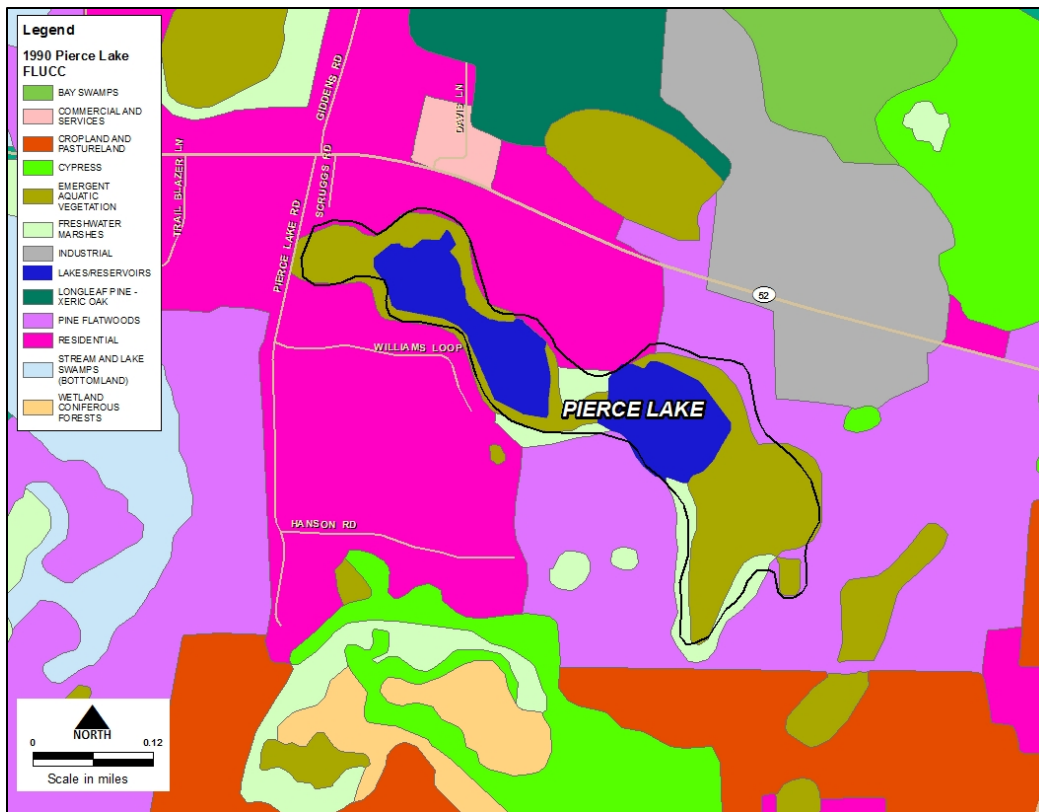


Figure 4: 1990 Land Use Land Cover Map of the Pierce Lake Vicinity.

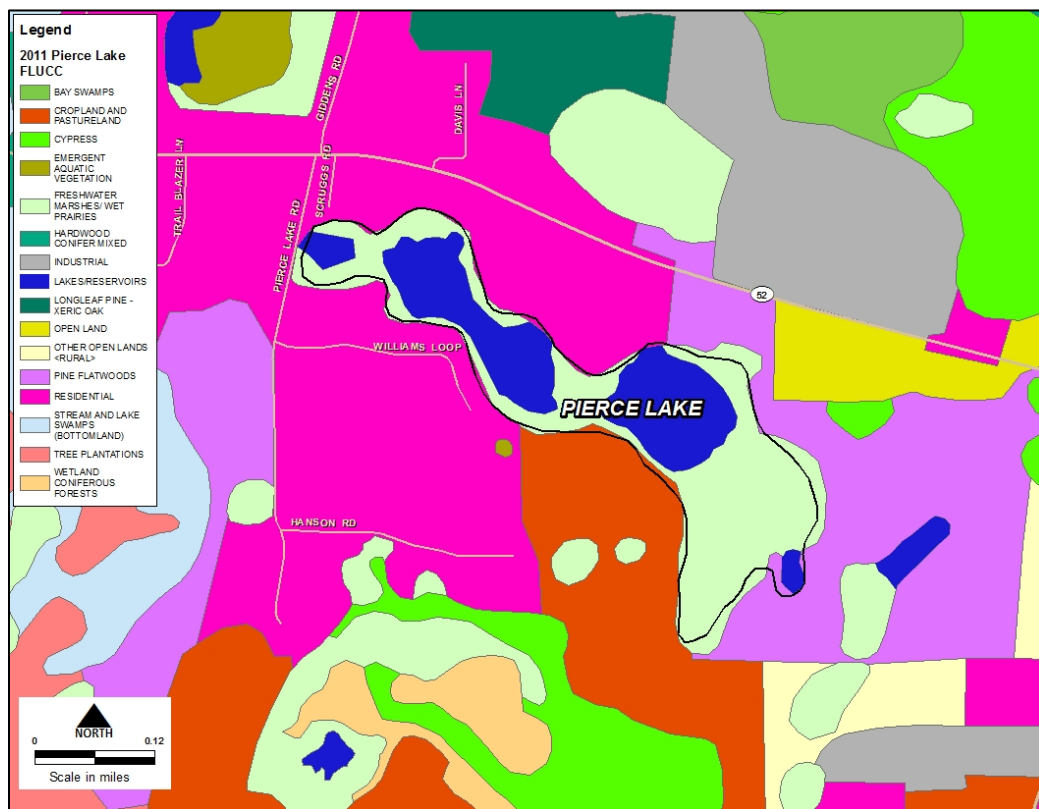


Figure 5: 2011 Land Use Land Cover Map of the Pierce Lake Vicinity.



Figure 6: 1941 Aerial Photograph of Pierce Lake



Figure 7: 1957 Aerial Photograph of Pierce Lake

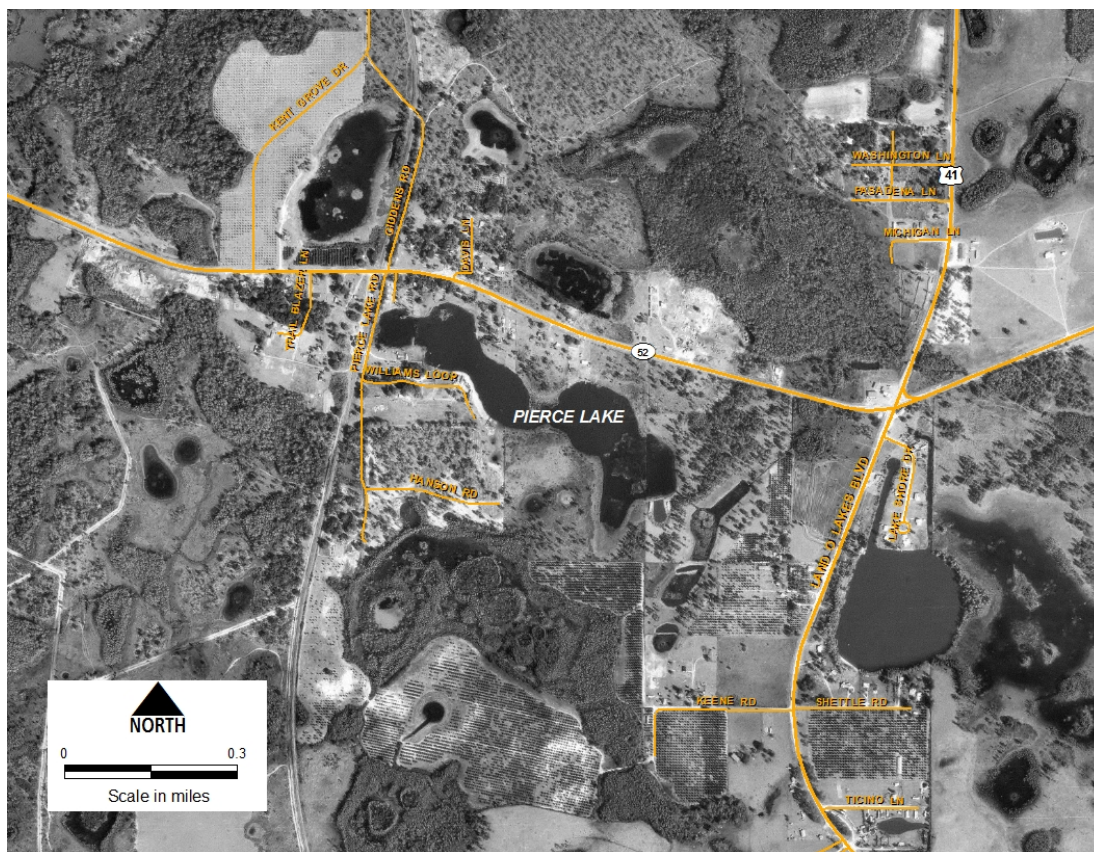


Figure 8: 1973 Aerial Photograph of Pierce Lake



Figure 9: 1996 Aerial Photograph of Pierce Lake



Figure 10: 2000 Aerial Photograph of Pierce Lake

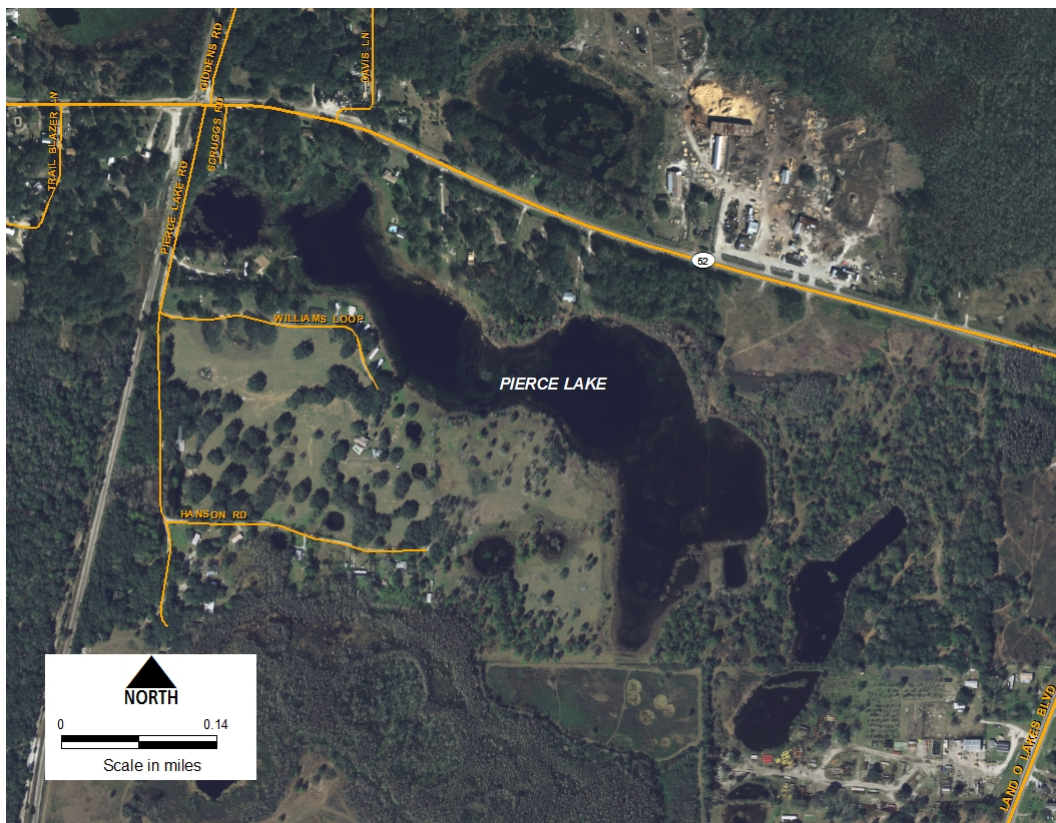


Figure 11: 2006 Aerial Photograph of Pierce Lake



Figure 12: 2007 Aerial Photograph of Pierce Lake



Figure 13a: 2012 Aerial Photograph of Pierce Lake



Figure 13b: 2012 Aerial Photograph of Pierce Lake

Bathymetry Description and History

More than 3,000 bathymetric data points gathered from field surveys resulted in lake-bottom contour lines from 55.47 ft. to 73.97 ft., NGVD29 (Figure 14). These data revealed that the lowest lake bottom contour, or the deepest part of the lake, is located in the eastern-most lobe of the lake. Additional morphometric or bathymetric information for the lake basin is discussed in the Methods, Results and Discussion section of this report.

Water Level (Lake Stage) Record

Lake stage data, i.e., surface water elevations, are available for Pierce Lake from the District's Water Management Information System (SID 20426) (Figure 15). Data collection began on April 27, 1981. Water elevations continue to be monitored on a monthly basis at the time of this report. On November 6, 2014 the gauge was adjusted from NGVD29 to NAVD88, with a measured shift of -0.92 ft (elevations in this document continue to be reported in feet NGVD29, unless otherwise noted). The highest lake stage elevation on record was 73.6 ft. and occurred on March 20, 1998. The lowest lake stage elevation on record was 65.52 ft. and occurred on June 13, 2000.

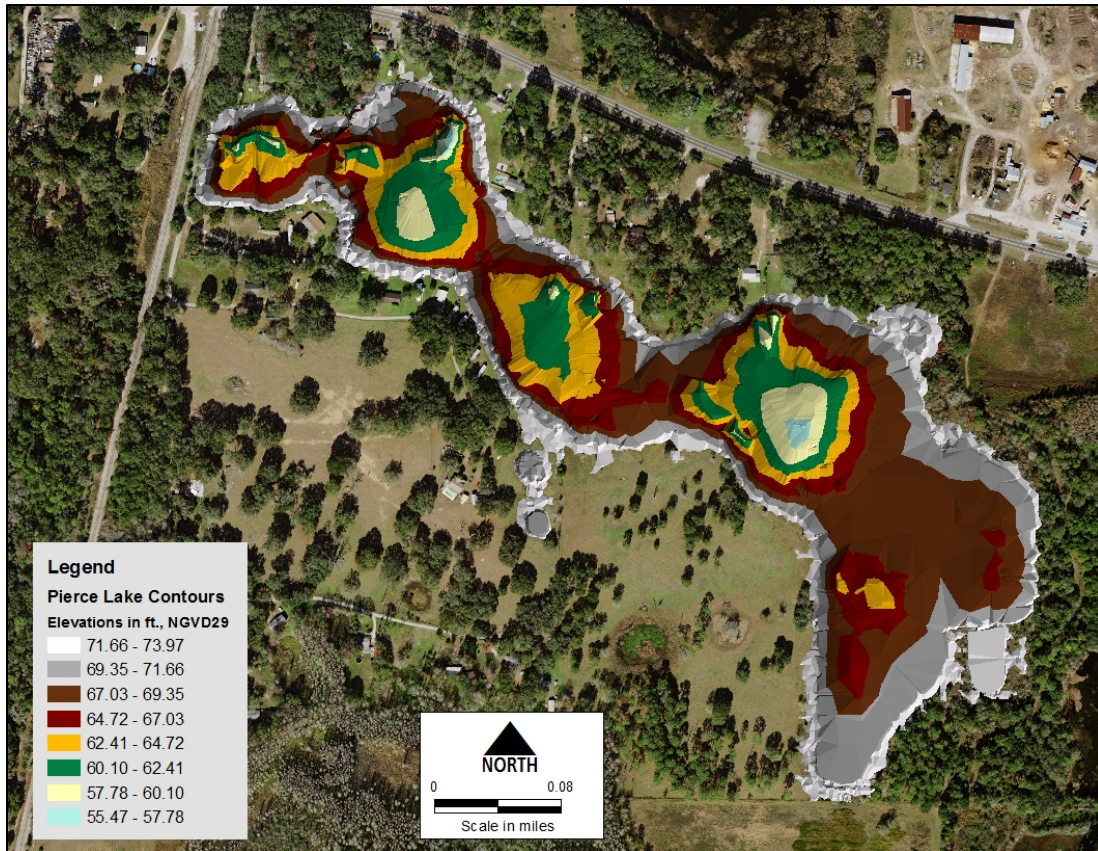


Figure 14: Lake Bottom Contours (ft., NGVD29) on a 2017 Natural Color Aerial Photograph

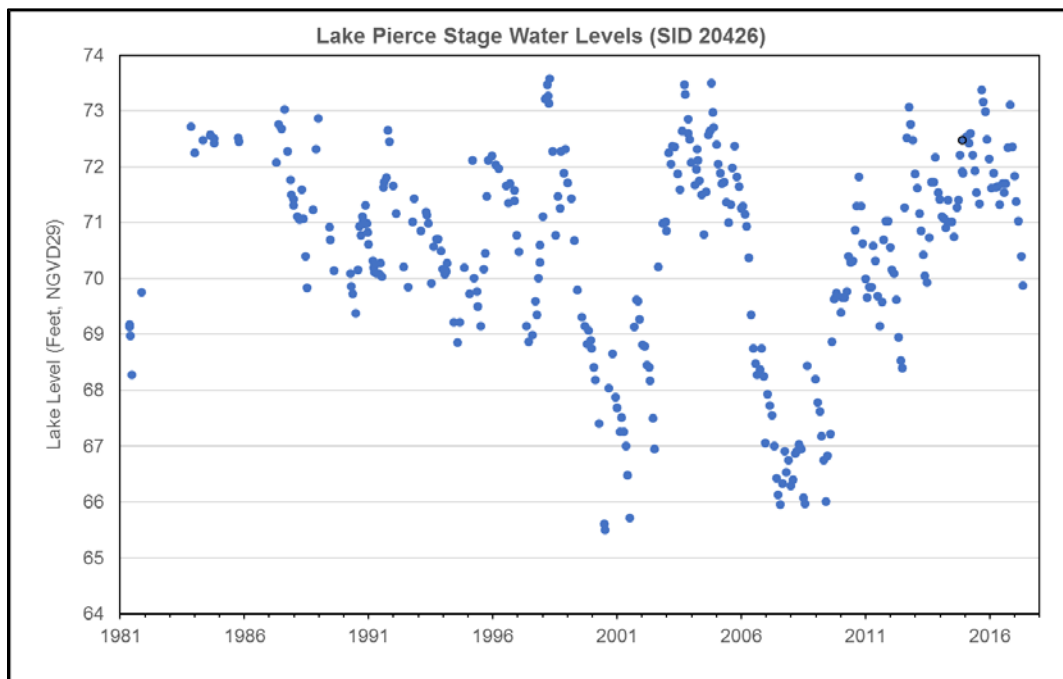


Figure 15: Pierce Lake Period of Record Water Elevation Data (SID 20426)

Historic Management Levels

The District has a long history of water resource protection through the establishment of lake management levels. With the development of the Lake Levels Program in the mid-1970s, the District began establishing management levels based on hydrologic, biological, physical, and cultural aspects of lake ecosystems. By 1996, management levels for nearly 400 lakes had been adopted into District rules.

The District Governing Board first approved Guidance and Minimum Levels for Pierce Lake (Table 2) in November 30, 2006, which were subsequently adopted into Chapter 40D-8, Florida Administrative Code, on May 20, 2008, using the methodology for Category 3 Lakes described in SWFWMD (1999a and 1999b).

Table 2: Minimum and Guidance Levels adopted May 20, 2008 for Pierce Lake

Level	Elevation (ft., NGVD)
High Guidance Level	72.7
High Minimum Level	72.2
Minimum Level	70.5
Low Guidance Level	68.9

Methods, Results and Discussion

The Minimum and Guidance Levels in this report were developed for Pierce Lake using the methodology for Category 3 lakes described in Chapter 40D-8, F.A.C. Levels, Standards, and other information used for development of the levels, are listed in Table 3, along with lake surface area for each level. Detailed descriptions of the development and use of these data are provided in subsequent sections of this report.

Table 3: Lake Stage Percentiles, Normal Pool and Control Point Elevations, Significant Change Standards, and Minimum and Guidance Levels with associated surface areas for Pierce Lake.

Levels	Elevation in Feet NGVD 29	Lake Area (acres)
Lake Stage Percentiles		
Historic P10 (1946 to 2018*)	72.7	43.8
Historic P50 (1946 to 2018*)	70.6	34.0
Historic P90 (1946 to 2018*)	68.5	26.1
Normal Pool and Control Point		
Normal Pool	NA	NA
Control Point	72.6	43.2
Significant Change Standards*		
Recreation/Ski Standard	NA	NA
Dock-Use Standard	71.5	37.0
Wetland Offset Elevation	69.8	31.4
Aesthetics Standard	68.5	26.1
Species Richness Standard	69.1	29.2
Basin Connectivity Standard	70.9	35.0
Lake Mixing Standard	NA	NA
Minimum and Guidance Levels		
High Guidance Level	72.7	43.8
High Minimum Lake Level	71.9	39.2
Minimum Lake Level	69.8	31.4
Low Guidance Level	68.5	26.1

NA - not appropriate * May 2018

Bathymetry

Relationships between lake stage, inundated area, and volume can be used to evaluate expected fluctuations in lake size that may occur in response to climate, other natural factors, and anthropogenic impacts such as structural alterations or water withdrawals. Long term reductions in lake stage and size can be detrimental to many of the environmental values identified in the Water Resource Implementation Rule for consideration when establishing MFLs. Stage-area-volume relationships are therefore useful for developing significant change standards and other information identified in District rules for consideration when developing minimum lake levels. The information is also needed for the development of lake water budget models that estimate the lake's response to rainfall and runoff, outfall or discharge, evaporation, leakage, and groundwater withdrawals.

Stage-area-volume relationships were determined for Pierce Lake by building and processing a digital elevation model (DEM) of the lake basin and surrounding watershed. Elevations of the lake bottom and land surface elevations were used to build the model through a series of analyses using LP360 (by QCoherent) for ArcGIS, ESRI® ArcMap 10.2 software, the 3D Analyst ArcMap Extension, Python, and XTools Pro. The overall process involves merging the terrain morphology of the lake drainage basin with the lake basin morphology to develop one continuous 3D digital elevation model. The 3D digital elevation model is then used to calculate area of the lake and the associated volume of the lake at different elevations, starting at the largest size of the lake at its peak or flood stage, and working downward to the base elevation (deepest pools in the lake).

Two elevation data sets were used to develop the terrain model for Pierce Lake. Light Detection and Ranging Data (LiDAR) was processed with LP360 for ArcGIS and merged with bathymetric data collected with both sonar and mechanical (manual) methods. These data were collected using a LEI HS-WSPK transducer (operating frequency = 192kHz, cone angle = 20) mounted to a boat hull, a Lowrance LMS-350A sonar-based depth finder and the Trimble GPS Pathfinder Pro XR/Mapping System (Pro XR GPS Receiver, Integrated GPS/MSK Beacon Antenna, TDC1 Asset Surveyor and Pathfinder Office software).

The DEM created from the combined elevation data sets was used to develop topographic contours of the lake basin and to create a triangulated irregular network (TIN). The TIN was used to calculate the stage areas and volumes using a Python script file to iteratively run the Surface Volume tool in the Functional Surface toolset of the ESRI® 3D Analyst toolbox at one-tenth of a foot elevation change increments. Selected stage-area-volume results are presented in Figure 16.

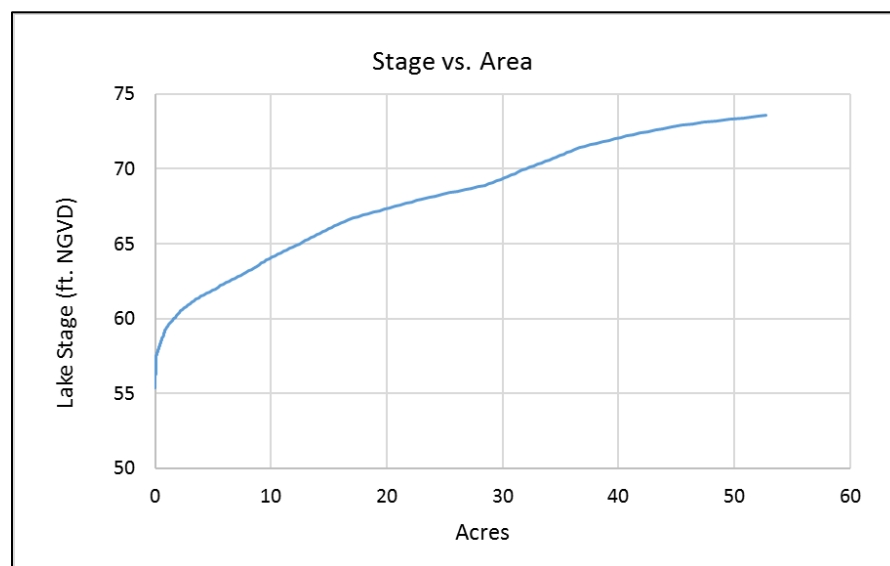


Figure 16: Lake Stage (Ft. NGVD29) to Surface Area (Acres) for Pierce Lake.

Development of Exceedance Percentiles

A key part of establishing Minimum and Guidance Levels is the development of exceedance percentiles based on Historic water levels (lake stage data). For the purpose of minimum levels determination, lake stage data are categorized as "Historic" for periods when there were no measurable impacts due to water withdrawals and impacts due to structural alterations were similar to existing conditions. In the context of minimum levels development, "structural alterations" means man's physical alteration of the control point, or highest stable point along the outlet conveyance system of a lake, to the degree that water level fluctuations are affected.

Based on water-use estimates and analysis of lake water levels and regional ground water fluctuations, a modeling approach (see Appendix A) was used to estimate Historic lake levels. This approach was considered appropriate for extending the period of record for lake stage values for developing Historic lake stage exceedance percentiles. Development of this stage record was considered necessary for characterization of the range of lake-stage fluctuations that could be expected based on long-term climatic cycles that have been shown to be associated with changes in regional hydrology (Enfield et al. 2001, Basso and Schultz 2003, Kelly 2004).

The initial approach included developing a water budget model which incorporated the effects of precipitation, evaporation, overland flow, and groundwater interactions (Appendix A). Using the results of the water budget model, regression modeling for lake stage predictions was conducted using a linear line of organic correlation statistical model (LOC) (see Helsel and Hirsch 1992). The procedure was used to derive the relationship between daily water surface elevations for Pierce Lake and composite regional rainfall.

A combination of model data produced a hybrid model which resulted in a 72-year (1946-2018) Historic water level record. Based on this hybrid data, the Historic P10 elevation, i.e., the elevation of the lake water surface equaled or exceeded ten percent of the time, was 72.7 ft. The Historic P50, the elevation the lake water surface equaled or exceeded fifty percent of the time during the historic period was 70.6 ft. The Historic P90, the lake water surface elevation equaled or exceeded ninety percent of the time during the historic period, was 68.5 ft. (Figure 17 and Table 3).

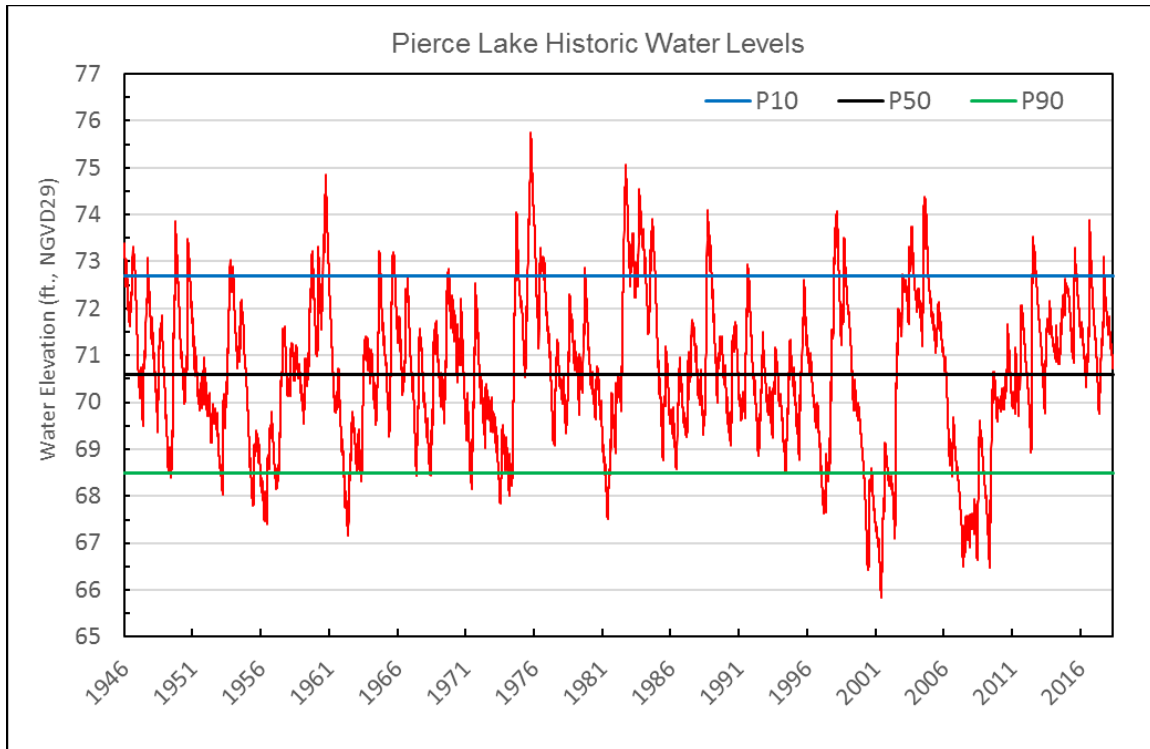


Figure 17: Historic Water Levels (hybrid model) Used to Calculate Percentile Elevations Including P10, P50, and P90.

Normal Pool Elevation and Additional Information

The Normal Pool elevation, a reference elevation used for development of minimum lake and wetland levels, is established based on the elevation of hydrologic indicators of sustained inundation. The inflection points (buttress swelling) and moss collars on the trunks of cypress trees have been shown to be reliable biologic indicators of hydrologic Normal Pool (Carr et al. 2006). As Pierce Lake does not have sufficient cypress trees with adequate hydrologic indicators, a Normal Pool elevation was not determined.

Additional information to consider in establishing Minimum and Guidance Levels are the Control Point elevation and the lowest building floor (slab) elevation within the lake basin (determined by field survey data). The Control Point elevation is the elevation of the highest stable point along the outlet profile of a surface water conveyance system that can principally control the lake water level fluctuations at the high end. The Control Point for Pierce Lake was determined at 72.6 ft., the elevation of the southern-most edge of the lake. The low floor slab elevation, based on survey reports, was established at 75.1 ft.

Guidance Levels

The High Guidance Level (HGL) is provided as an advisory guideline for construction of lakeshore development, water dependent structures, and operation of water management structures. The High Guidance Level is the expected Historic P10 of the

lake and is established using Historic data if it is available, or is estimated using the Current P10, the Control Point elevation and the Normal Pool elevation. Based on the availability of Historic data developed for Pierce Lake, the High Guidance Level was established at the Historic P10 elevation, 72.7 ft. Recorded stage data indicate that the highest water levels were reached in March 1998 (73.6 ft.), August 2003 (73.5 ft.) and 2004 (73.5 ft.).

The Low Guidance Level (LGL) is provided as an advisory guideline for water dependent structures, and as information for lakeshore residents and operation of water management structures. The Low Guidance Level is the elevation that a lake's water levels are expected to equal or exceed ninety percent of the time on a long-term basis. The level is established using Historic or Current lake stage data and, in some cases, Reference Lake Water Regime (RLWR) statistics. Based on the availability of Historic data for Pierce Lake, the Low Guidance Level was established at the Historic P90 elevation, 68.5 ft. Recorded stage data indicate that the lowest water levels were reached in June 2000 (65.5 ft.), July 2007 (66.0 ft.) and July 2008 (66.0 ft.).

Significant Change Standards

Category 3 significant change standards were determined for Pierce Lake based on the stage-area-volume relationship which was developed. These standards include a Recreation/Ski Standard, Dock-Use Standard, Wetland Offset Elevation, Aesthetics Standard, Species Richness Standard, Basin Connectivity Standard, and Lake Mixing Standard. Each standard was evaluated for minimum levels development for Pierce Lake and presented in Table 3.

- The **Recreation/Ski Standard** was not established due to the shallowness of the water within a circular ski corridor with a radius of 418 feet or a rectangular corridor 200 x 2,000 feet. Thus, Pierce Lake is classified as a Non-Ski lake.
- The **Dock-Use Standard** was established at an elevation of 71.5 ft. based on the elevation of lake sediments at the end of 3 docks on the lake, a 2-ft. clearance depth, and the difference between the Historic P50 and P90 of 2.1 ft.
- The **Wetland Offset Elevation** was established at 69.8 ft., or 0.8 ft. below the historic P50 elevation.
- The **Aesthetic Standard** was established at the Low Guidance Level elevation of 68.5 ft.
- The **Species Richness Standard** was established at 69.1 ft., based on a 15% reduction in lake surface area from that at the Historic P50 elevation.
- The **Basin Connectivity Standard** was established at an elevation of 70.9 ft. based on a critical high spot elevation of 67.8 ft, the addition of 2 feet, plus the difference between the Historic P50 and P90 of 2.1 ft. This critical high spot is the elevation separating the center and eastern-most “pools” of Pierce Lake.

- The **Lake Mixing Standard** was not established, as the dynamic ratio does not reach a value of 0.8 (see Bachmann et al. 2000).
- Review of changes in potential **herbaceous wetland** area associated with change in lake stage (Figure 18), and potential changes in area available for **aquatic plant colonization** (Figure 19) did not indicate that use of any of the identified standards would be inappropriate for minimum levels development. Figure 18 shows that as the lake stage increases, the acres available for herbaceous wetland area (acres < 4 ft.) also increase, up until around 70.8 ft. NGVD. The acres available for herbaceous wetlands then stays constant up to 72 ft. and continues to increase to the lake's edge. The changes in the slope of the lines reflects the variation in lake bottom contours and the area which it contains. The area available for aquatic plant colonization (acres ≤ 9.2 ft.) follows a continual upward trend (Figure 19).

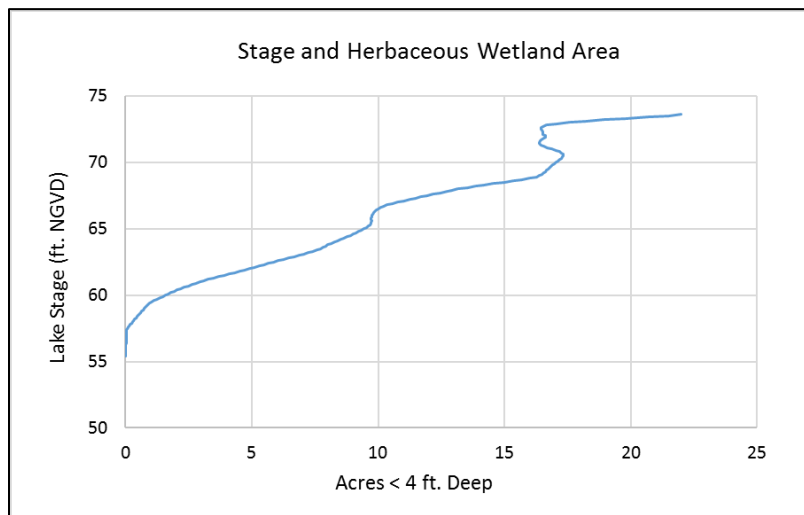


Figure 18: Lake Stage Compared to Available Herbaceous Wetland Area.

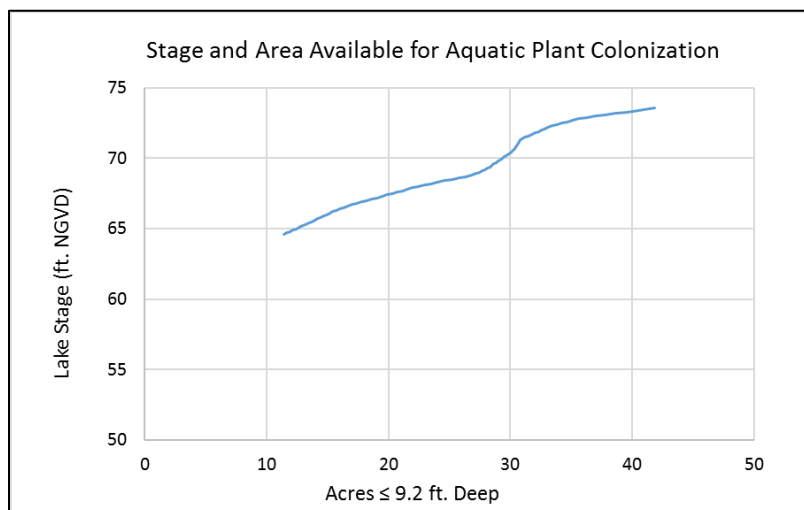


Figure 19: Lake Stage and Area Available for Aquatic Plant Colonization.

Minimum Levels

The Minimum Lake Level (MLL) is the elevation that a lake's water levels are **required** to equal or exceed fifty percent of the time on a long-term basis. For a Category 3 lake, the Minimum Lake Level is established using a process that considers applying professional experience and judgement, and the Standards previously listed. The MLL for Pierce Lake is established at the Wetland Offset elevation of 69.8 ft.

The High Minimum Lake Level (HMLL) is the elevation that a lake's water levels are **required** to equal or exceed ten percent of the time on a long-term basis. For a Category 3 lake, Rule 40D-8.624, F.A.C. allows for the HMLL to be established using one of two methods. The High Minimum Lake Level is established at the elevation corresponding to the Minimum Lake Level plus the difference between the Historic P10 and the Historic P50, or alternatively, the HMLL is established at the elevation corresponding to the MLL plus the RLWR value. Due to the availability of Historic percentiles, the HMLL was established using the first method, resulting in a HMLL of 71.9 ft. This elevation accounts for a natural fluctuation of lake levels.

Minimum and Guidance levels for Pierce Lake are plotted on the recorded water level record in Figure 20. To illustrate the approximate locations of the lake margin when water levels equal the minimum and guidance levels, the levels are imposed onto a 2017 natural color aerial photograph in Figure 21.

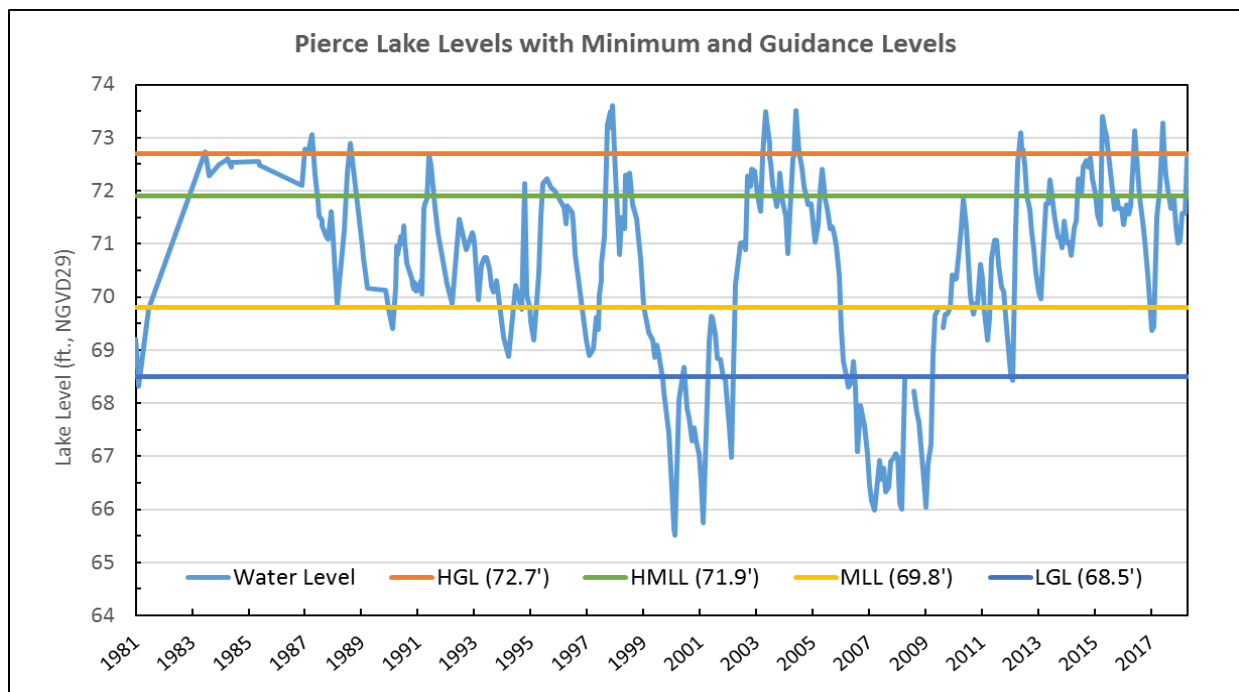


Figure 20: Recorded Water Level Elevations with Guidance and Minimum Lake Levels for Pierce Lake

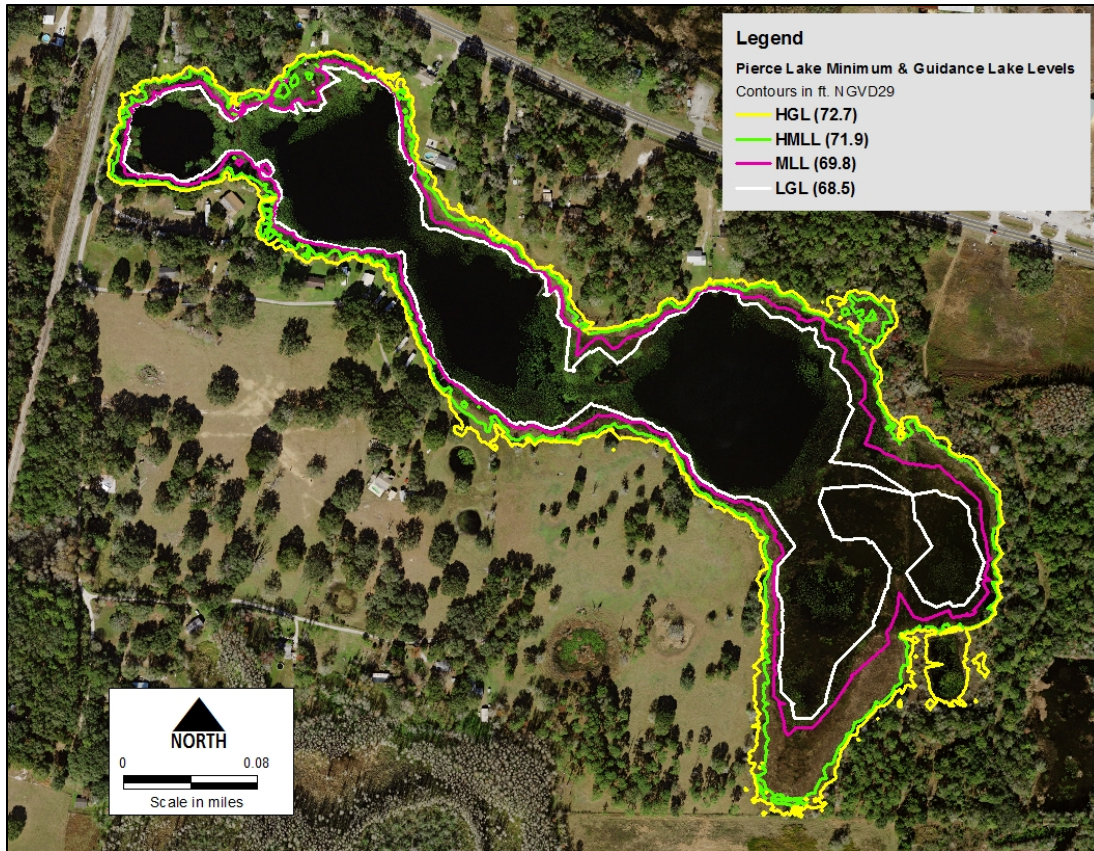


Figure 21: Pierce Lake Minimum and Guidance Level Contour Lines Imposed onto a 2017 Natural Color Aerial Photograph.

Many federal, state, and local agencies, such as the U.S. Army Corps of Engineers, the Federal Emergency Management Agency, United States Geological Survey, and Florida's water management districts are in the process of upgrading from the National Geodetic Vertical Datum (NGVD29) standard to the North American Vertical Datum (NAVD88) standard. For comparison purposes, the MFLs for Pierce Lake are presented in both datum standards (Table 4). The datum shift was calculated based on third-order leveling ties from vertical survey control stations with known elevations above the North American Vertical Datum on 1988. The NGVD29 datum conversion to NAVD88 is -0.92 ft. for SID 20426 on Pierce Lake.

Table 4: Minimum and Guidance Levels for Pierce Lake in NGVD29 and NAVD88.

Minimum and Guidance Levels	Elevation in Feet NGVD29	Elevation in Feet NAVD88
High Guidance Level	72.7	71.78
High Minimum Lake Level	71.9	70.98
Minimum Lake Level	69.8	68.88
Low Guidance Level	68.5	67.58

Consideration of Environmental Values

The minimum levels for Pierce Lake are protective of relevant environmental values identified for consideration in the Water Resource Implementation Rule when establishing minimum flows and levels (see Rule 62-40.473, F.A.C.). As presented above, when developing minimum lake levels, the District evaluates categorical significant change standards and other available information to identify criteria that are sensitive to long-term changes in hydrology and represent significant harm thresholds.

The Wetland Offset Elevation was used for developing Minimum Levels for Pierce Lake based on its classification as a Category 3 lake. This standard is associated with protection of several environmental values identified in Rule 62-40.473, F.A.C., including: fish and wildlife habitats and the passage of fish, transfer of detrital material, aesthetic and scenic attributes, filtration and absorption of nutrients and other pollutants, and water quality (Table 1).

In addition, the environmental value of maintenance of freshwater storage and supply is also expected to be protected by the minimum levels based on inclusion of conditions in water use permits that stipulate permitted withdrawals will not lead to violation of adopted minimum flows and levels.

Two environmental values identified in the Water Resource Implementation Rule were not considered relevant to development of minimum levels for Pierce Lake. Estuarine resources were not considered relevant because the lake is not connected to an estuarine resource. Sediment loads were similarly not considered relevant for minimum levels development for the lake, because the transport of sediments as bedload or suspended load is a process typically associated with flowing water systems.

Comparison of Revised and Previously Adopted Levels

The High Guidance Level is the same as the previously adopted High Guidance Level, while the Low Guidance Level is 0.4 feet lower than the previously adopted Low Guidance Level (Table 6). The differences in the Low Guidance level is associated with application of a new modeling approach for characterization of Historic water level fluctuations within the lake, i.e., water level fluctuations that would be expected in the absence of water withdrawal impacts given existing structural conditions, and additional data since the last evaluation.

The High Minimum Lake Level for Pierce Lake is 0.3 feet lower from the previously adopted High Minimum Lake Level and the Minimum Lake Level is 0.7 feet lower than the previously adopted Minimum Lake Level (Table 6). These differences are due to the same factors discussed above for the changes in the Guidance Levels, as well as the fact that the MLL is based on the Wetland Offset. The previously adopted MLL was based on the Dock Standard which is now calculated to be above the Historic P50, and thus ruled out as the MLL.

The Minimum and Guidance Levels identified in this report replace the previously adopted levels for Pierce Lake.

Table 4: Minimum and Guidance Levels for Pierce Lake compared to previously adopted Minimum and Guidance Levels.

Minimum and Guidance Levels	Elevations (in Feet NGVD29)	Previously Adopted Elevations (in Feet NGVD29)
High Guidance Level	72.7	72.7
High Minimum Lake Level	71.9	72.2
Minimum Lake Level	69.8	70.5
Low Guidance Level	68.5	68.9

Minimum Levels Status Assessment

To assess if the Minimum and High Minimum Lake Levels are being met, observed stage data in Pierce Lake were used to create a long-term record using a Line of Organic Correlation (LOC) model, similar to what was developed for establishing the Minimum Levels (Appendix A). For the status assessment, the lake stage data used to create the LOC must be from a period representing a time when groundwater withdrawals and structural alterations are reasonably stable, and represent current conditions, referred to as the “Current” period. Current stage data observed on Pierce Lake were determined to be from 1971 through 2018. Using the Current stage data, the LOC model was created. The LOC model resulted in a 72-year long-term water level record (1946-2018).

For the status assessment, Long-term median (P50) and P10 water elevations were compared to the Minimum Lake Level and High Minimum Lake Level, respectively, to determine if long-term water levels were above these levels. Results from these assessments indicate that Pierce Lake water levels are above both the Minimum Lake Level and the High Minimum Lake Level (see Appendix B).

The lake lies within the region of the District covered by an existing recovery strategy for the Northern Tampa Bay Water Use Caution Area (Rule 40D-80.073, F.A.C.). The District plans to continue regular monitoring of water levels in Pierce Lake and will also routinely evaluate the status of the lake’s water levels with respect to adopted minimum levels for the lake included in Chapter 40D-8, F.A.C.

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APPENDIX A

Technical Memorandum

August 31, 2018

TO: Mark K. Hurst, Senior Environmental Scientist, Water Resources Bureau
David C. Carr, Staff Environmental Scientist, Water Resources Bureau

THROUGH: Tamera McBride, P.G, Manager, Resource Evaluation, Water Resources Bureau

FROM: Cortney Cameron, G.I.T., Hydrogeologist, Water Resources Bureau
Michael C. Hancock, P.E., Chief Prof. Engineer, Water Resources Bureau

Subject: Pierce Lake Water Budget Model, Rainfall Correlation Model, and Historic Percentile Estimations

A. Introduction

Water budget and rainfall correlation models were developed to assist the Southwest Florida Water Management District (District) in the reassessment of minimum levels for Pierce Lake in west-central Pasco County. Pierce Lake currently has adopted minimum levels which are scheduled to be re-assessed in FY 2018. This document will discuss the development of the Pierce Lake models and use of the models for development of Historic lake stage exceedance percentiles.

B. Background and Setting

Pierce Lake is in west-central Pasco County, southwest of the intersection between State Road 52 and U.S. Highway 41 (Figure 1). The lake lies within the Gowers Corner Slough basin that forms part of the larger Crystal-Pithlachascotee (also called the Upper Coastal) watershed (USGS HUC 03100207). Pierce Lake has no significant inflow other than overland flow. At stages above approximately 72.6 ft NGVD29, Pierce Lake discharges through a ditch at its southern end across a field; additionally, at stages above approximately 73.4 ft NGVD29 (which occurs infrequently), Pierce Lake also discharges across a low spot in a pasture along its south-southwestern shore (Figure 2). Both outflows enter the same wetland to the lake's southwest. The topography is very flat, however, and flows are often negligible.

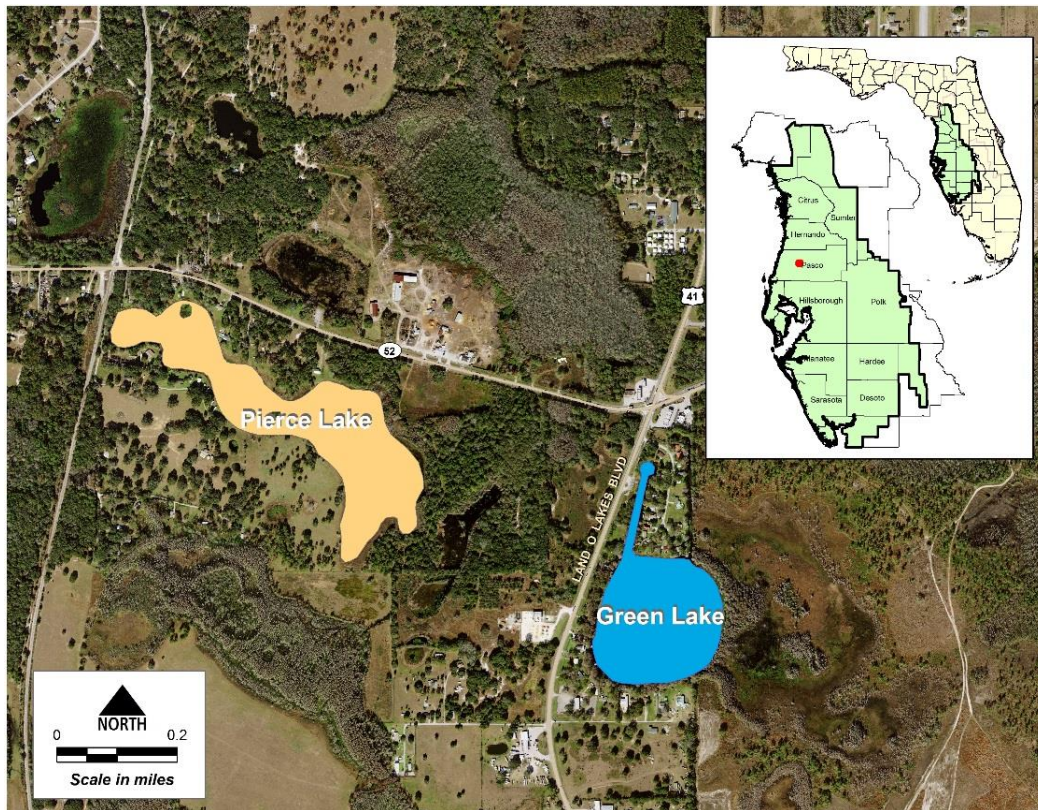


Figure 1. Location of Pierce Lake in Pasco County, Florida.

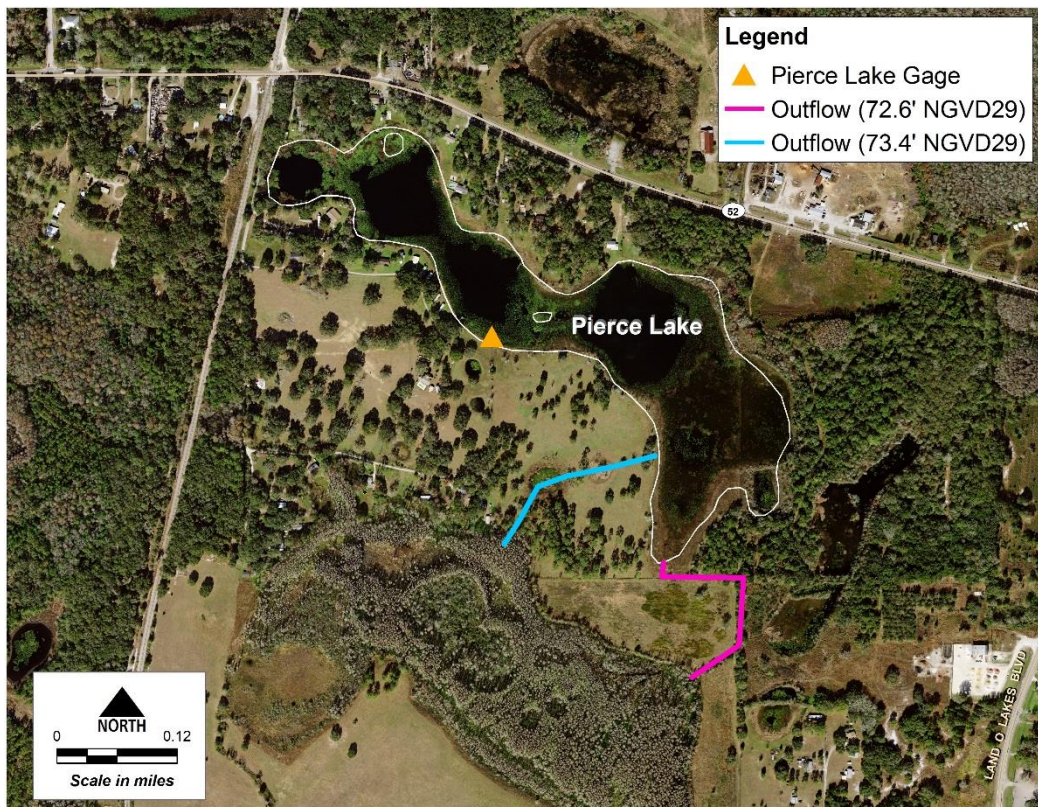


Figure 2. Flow out of Pierce Lake (pink and blue lines).

Physiography and Hydrogeology

White (1970) classified the region of Florida containing Pierce Lake as the Northern Gulf Coastal Lowlands. Brooks (1981) categorized the area surrounding the lake as the Land O' Lakes subdivision of the Tampa Plain division of the Ocala Uplift District, describing the region as a plain with numerous small lakes imbedded in moderately thick silty-sand deposits lying above the Tampa Limestone formation. The topography is very flat, and drainage into the lake is a combination of overland flow and flow through drainage swales and minor conveyance systems.

The hydrogeology of the area includes a sand surficial aquifer; a discontinuous, intermediate clay confining unit; and the thick carbonate Upper Floridan aquifer. In general, the surficial aquifer in the study area is in good hydraulic connection with the underlying Upper Floridan aquifer because the clay confining unit is generally thin, discontinuous, and breached by numerous karst features. The surficial aquifer is generally ten to thirty feet thick and overlies the limestone of the Upper Floridan aquifer that averages nearly one thousand feet thick in the area (Miller, 1986). In between these two aquifers is the Hawthorn Group clay that varies between a few feet to as much as 25 feet thick, being about 10 feet thick near Pierce Lake based on drill cutting logs near the lake and mapping from the Florida Geological Survey. Because the clay unit is breached by buried karst features and has previously been exposed to erosional processes, preferential pathways locally connect the overlying surficial aquifer to the Upper Floridan aquifer resulting in moderate-to-high leakage to the Upper Floridan aquifer (Hancock and Basso, 1996). Data from Cross Bar 2SW CSX well nest next to Pierce Lake suggests that surficial and Upper Floridan aquifer heads closely track each other, separating mainly during high rainfall periods (Figure 5); the average head difference from May 1997 to May 2018 was just 0.9 feet.

Data

The District began collecting water level data at Pierce Lake in April 1981 at a gage (SID 20426) on its central southwestern shore (Figures 2 and 3). Data collection occurred sporadically (a few times a year, with no data for 1982 and 1986) until March 1987, after which collection approached monthly, with variations and gaps through 2003, after which data became reliably monthly.

Due to their proximity to Pierce Lake, one Upper Floridan monitor well (SID 20427) and one surficial aquifer monitor well (SID 20428) were considered for the analysis (Figure 4). The Cross Bar 2SW CSX Floridan and surficial aquifer monitor wells are each located approximately 0.3 miles from the lake's centroid and approximately 0.2 miles from the southwestern lakeshore. Data for these wells begin in May 1997 with daily data collection, with occasional gaps, occurring through the present (Figure 5).

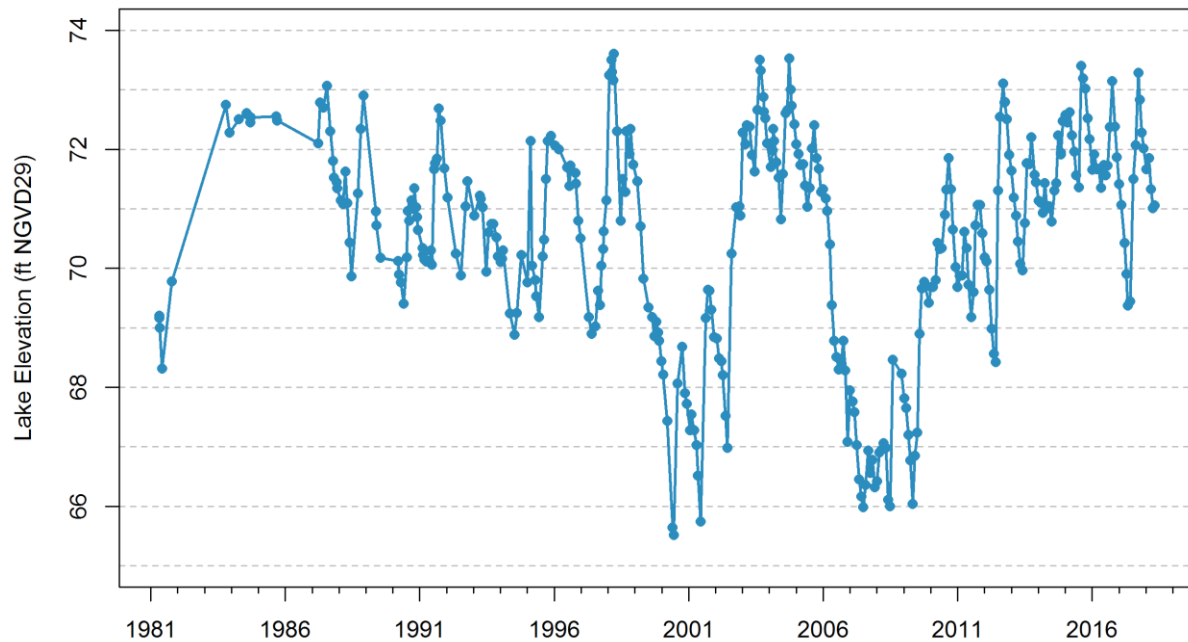


Figure 3. Pierce Lake water levels from 1981 to 2018.

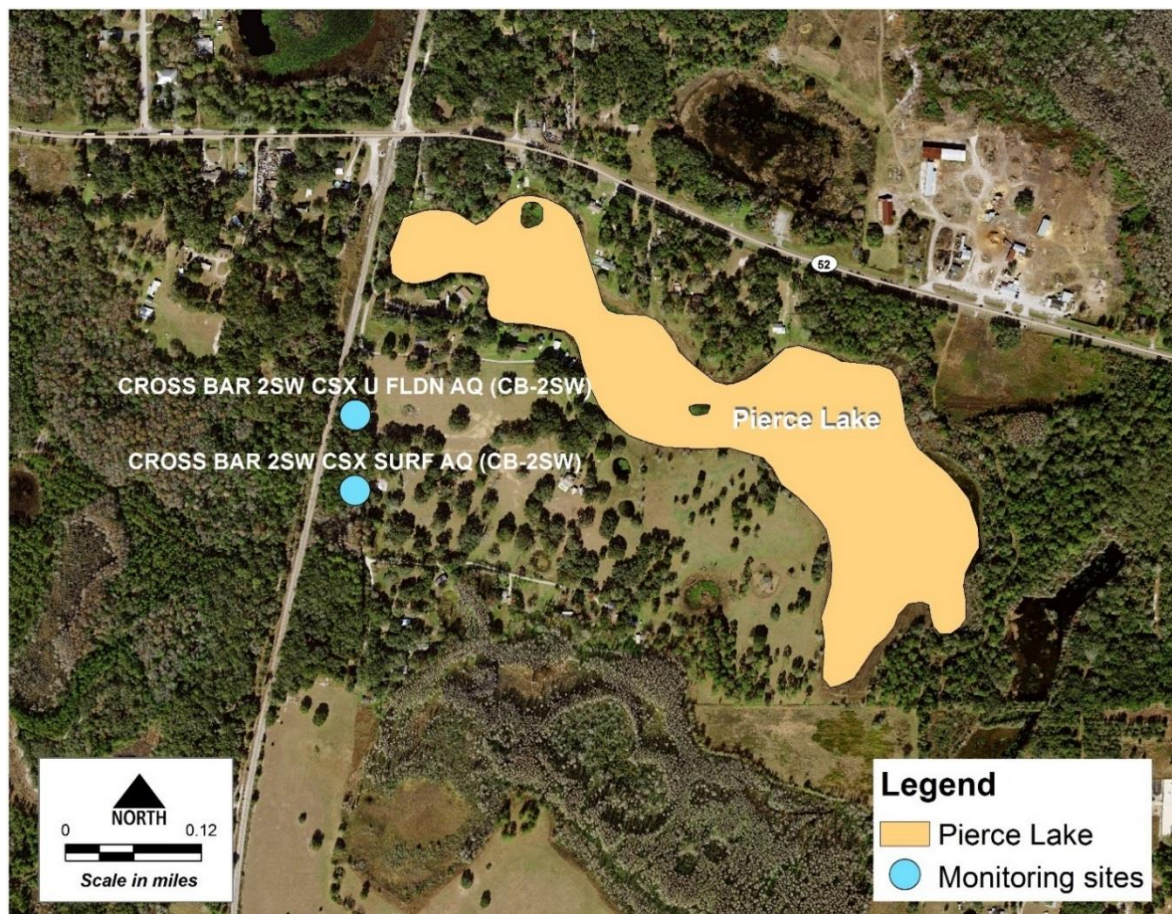


Figure 4. Location of monitor wells near Pierce Lake considered for model use.

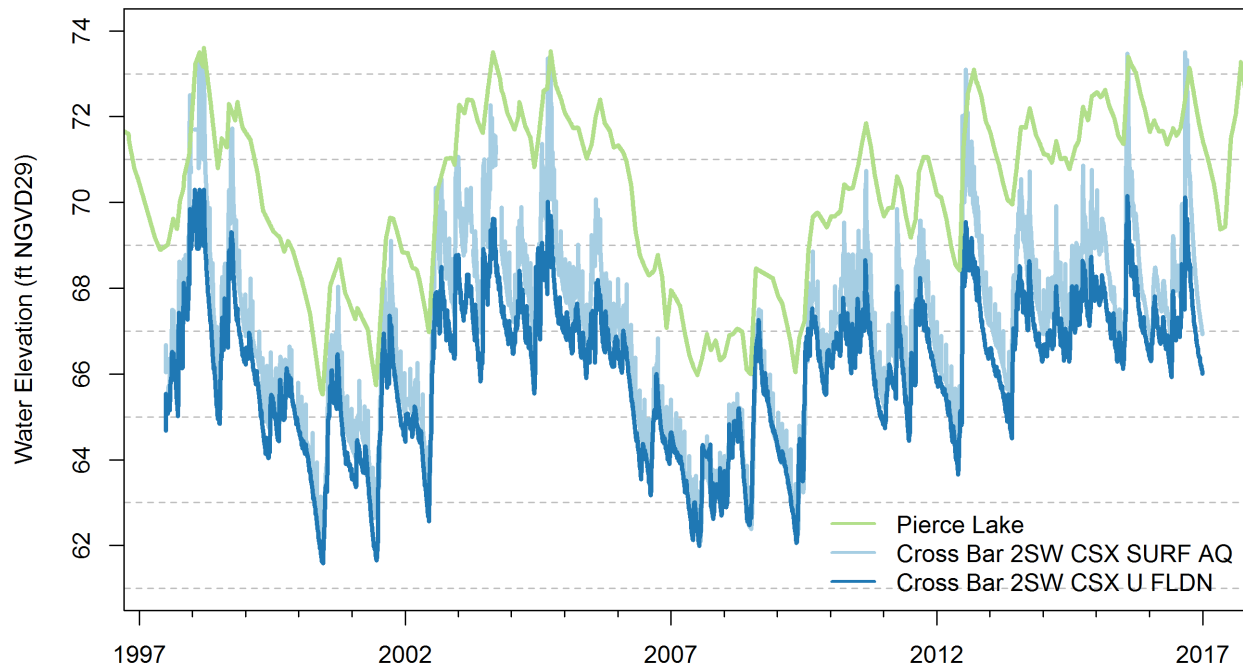


Figure 5. Water levels in Pierce Lake (green), Cross Bar 2SW CSX Surficial (light blue), and Upper Floridan (dark blue) aquifer monitor wells from 1997 to 2017.

Land and Water Use

Pierce Lake is located south-southwest of Cross Bar Ranch Wellfield, one of ten regional water supply wellfields operated by Tampa Bay Water (Figure 6). North Pasco Wellfield, to the west of Pierce Lake, discontinued operation in 2018. While Pierce Lake is located several miles away from the nearest regional water supply production well (North Pasco included), it nevertheless experienced some drawdown resulting from groundwater withdrawals (Appendix C). At the Cross Bar Ranch Wellfield, groundwater withdrawals began in 1980 at a rate of approximately 7 gallons per day (mgd), steadily climbing to a peak of 41 mgd by the end of 1993, after which it declined until cutbacks occurred in 2003, averaging 15 mgd thereafter. North Pasco Wellfield began operation in 1992, averaging 2 mgd until cutbacks in 2008, after which it averaged under 1 mgd until its closure in 2018.

Water levels in several lakes and wetlands near Cross Bar Ranch have dropped significantly since public supply groundwater withdrawals began in the area (Hancock and Basso, 1996). Because Pierce Lake water level data collection did not begin until well after the beginning of withdrawals from the wellfields (Figure 3 and 7), the correlation between groundwater withdrawals and lake levels is not easily seen in the early data. Lake recovery during the period of recent reductions in groundwater withdrawals can be seen in Figures 3 and 8, but above average rainfall during that period could also account for some of the apparent recovery.

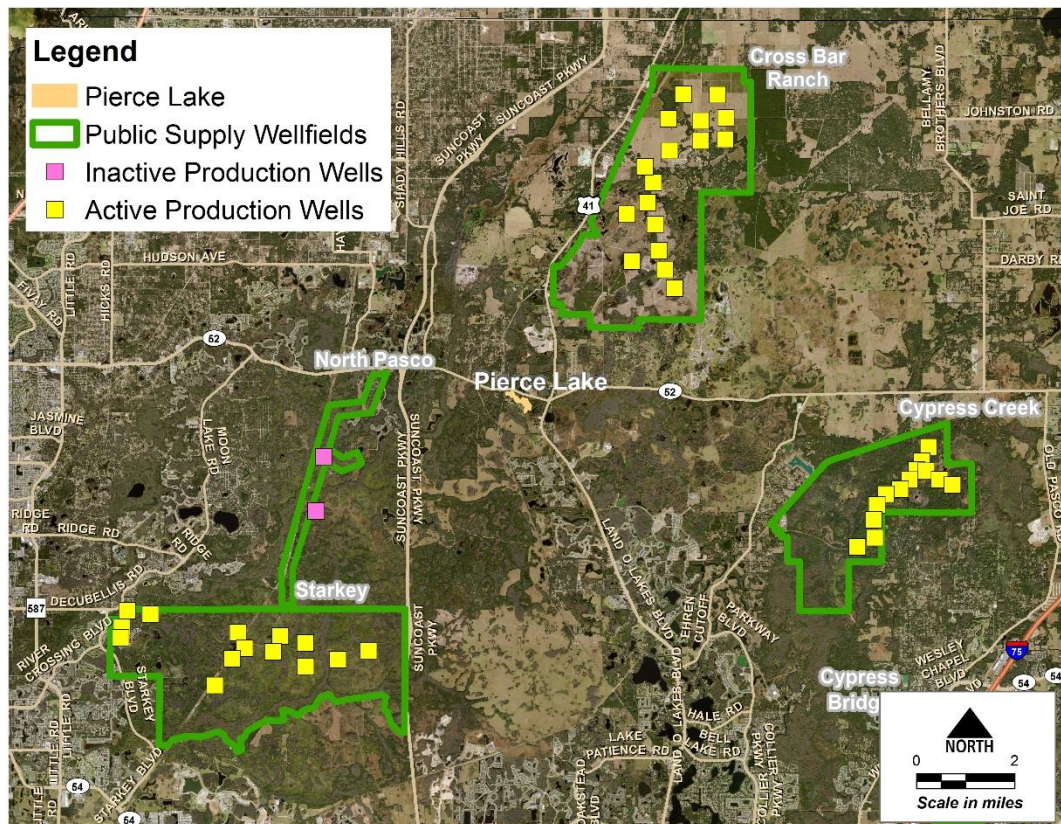


Figure 3. Pierce Lake (center) and (clockwise from Pierce) the Cross Bar Ranch, Cypress Creek, Starkey, and North Pasco wellfields.

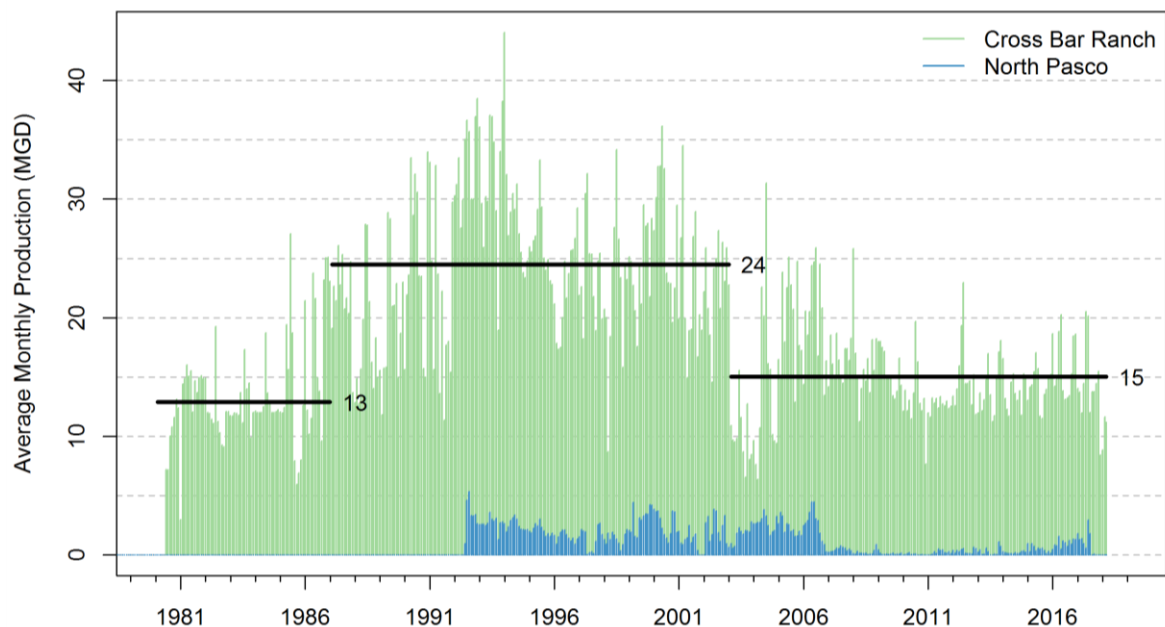


Figure 7. Stacked Cross Bar Ranch (red) and North Pasco (green) Wellfield average monthly withdrawals. Horizontal black lines indicate the average combined production (mgd) during the time period spanned by the line.

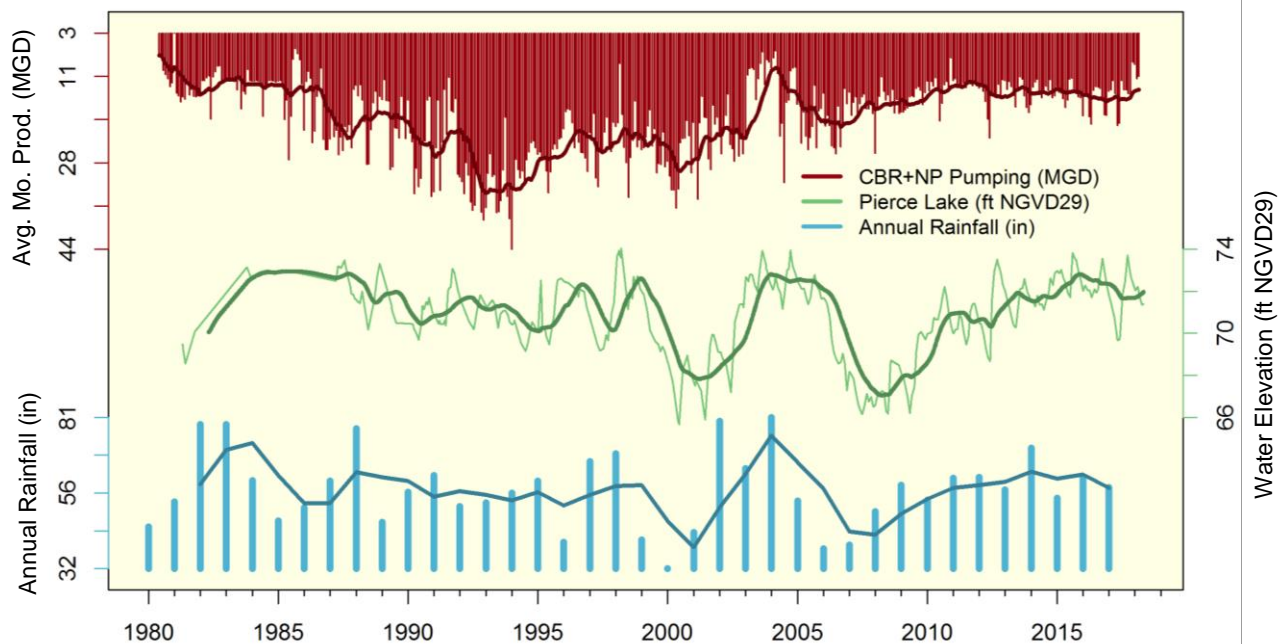


Figure 8. Water elevations in Pierce Lake (ft NGVD29; green, center), versus monthly average combined production at the Cross Bar Ranch and North Pasco Wellfields (MGD; red, top) and total annual rainfall (inches; blue, bottom). Thick lines represent one-year moving average for lake levels (green, center) and pumping (red, top) and three-year moving average for rain (blue, bottom). Note inverted y-axis for pumping.

Aerial and satellite imagery of Pierce Lake from 1941 to 2017 demonstrate that water levels have fluctuated considerably during that time (Figure 8). Following an extreme drought in 1956, the 1957 image shows lake bottom at the lake periphery, even prior to regional groundwater pumping in the vicinity of Pierce Lake.

The relationship between sinkhole formation or karst activity and hydrologic stress in the northwest Hillsborough County and surrounding areas has been well established and thoroughly discussed (Bredehoeft and others, 1965; Sinclair, 1973 Stewart and Hughes, 1974; Sinclair, 1982; Sinclair and others, 1985; Hancock and Basso, 1996; Metz and Sacks, 2002; and, Metz, 2011). Man-induced or natural hydrologic stress can cause sediments in karst formations to unravel or can lower water levels that support overburden covering voids in the limestone aquifer. This can result in sinkholes that appear on the surface, or can result in changes that occur underground and cannot be seen at the surface. These changes, in turn, can result in pathways for water to connect lakes, wetlands, or the surficial aquifer in general, to the underlying Upper Floridan aquifer. It is thus possible that a change in leakance properties between Pierce Lake and the Upper Floridan aquifer (possibly due to karst activity beneath or surrounding the lakes) has occurred.

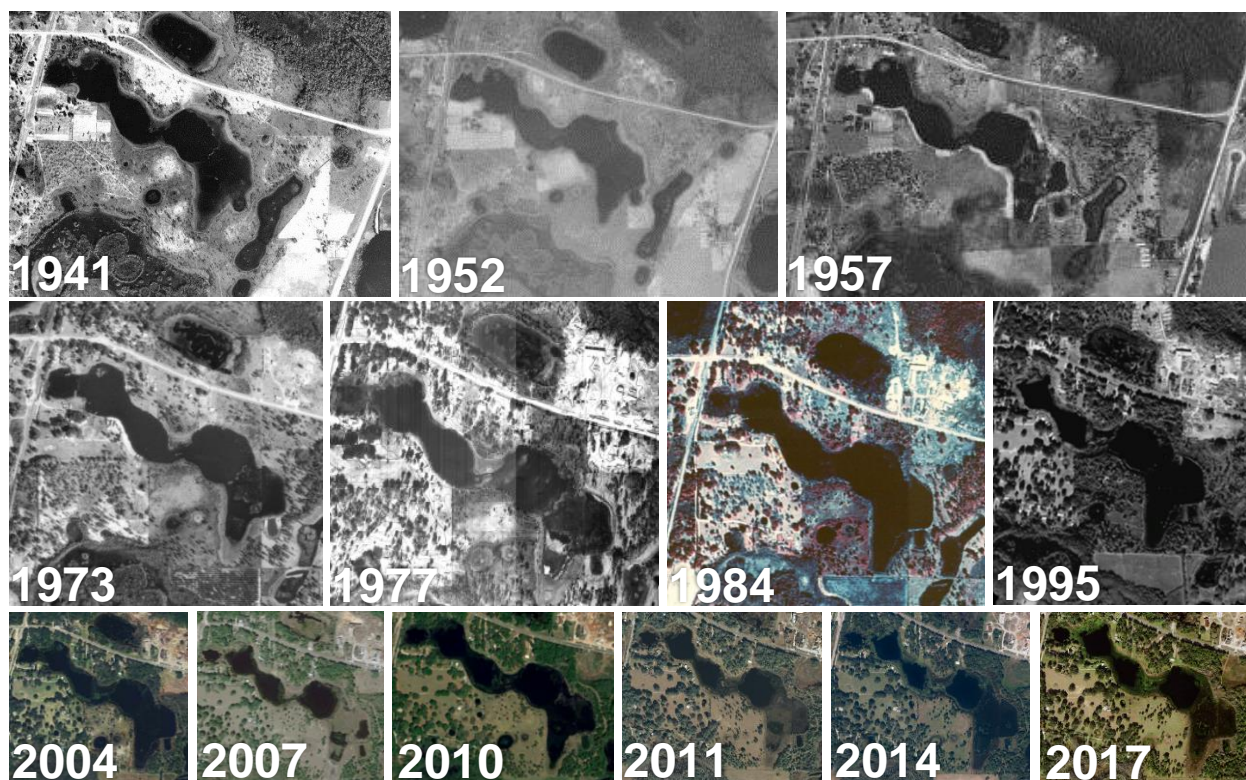


Figure 8. Water level changes in Pierce Lake.

C. Purpose of Models

Prior to establishment of Minimum Levels, long-term lake stage percentiles are developed to serve as the starting elevations for the determination of the lake's High Minimum Lake Level and the Minimum Lake Level. A critical task in this process is the delineation of a Historic time period. The Historic time period is defined as a period of time when there is little to no groundwater withdrawal impact on the lake, and the lake's structural condition is similar or the same as present day. The existence of data from a Historic time period is significant, since it provides the opportunity to establish strong predictive relationships between rainfall, groundwater withdrawals, and lake stage fluctuation that represent the lake's natural state in the absence of groundwater withdrawals. This relationship can then be used to calculate long-term Historic lake stage exceedance percentiles such as the P10, P50, and P90, which are, respectively, the water levels equaled or exceeded ten, fifty, and ninety percent of the time. If data representative of a Historic time period does not exist, or available Historic time period data is considered too short to represent long-term conditions, then a model is developed to approximate Long-term Historic data.

In the case of Pierce Lake, the Cross Bar Ranch Wellfield has potentially affected water levels since it began operation in 1980. As monitoring at Pierce Lake postdates the wellfield, empirical data are not available to evaluate the potential impacts of the early

groundwater withdrawals near the wellfields. Other groundwater withdrawals (including other wellfields) could also affect levels, but the effect of such withdrawals would be smaller and less consistent. Therefore, the development of a water budget model coupled with a rainfall correlation model of the lake was considered essential to estimate long-term Historic percentiles, accounting for any changes in the lake's drainage system, while simulating the effects of changing groundwater withdrawal rates.

D. Water Budget Model Overview

The Pierce Lake water budget model is a spreadsheet-based tool that includes natural hydrologic processes and engineered alterations acting on the control volume of the lake. The control volume consists of the free water surface within the lake extending down to the elevation of the greatest lake depth. A stage-volume curve was derived for the lake that produced a unique lake stage for any total water volume within the control volume.

The hydrologic processes in the water budget model include:

- a. Rainfall and evaporation
- b. Overland flow
- c. Inflow and discharge via channels
- d. Flow from and into the surficial aquifer
- e. Flow from and into the Upper Floridan aquifer

The water budget model uses a daily time-step, and tracks inputs, outputs, and lake volume to calculate a daily estimate of lake levels for the lake. The water budget model for Pierce Lake is calibrated from May 1997 through May 2018, with well data the temporally limiting factor. This period provides the best balance of using available data for all parts of the water budget and the desire to develop a long-term water level record. Model inputs are summarized in Table 1.

E. Water Budget Model Components

Lake Stage/Volume

Lake stage area and stage volume estimates were determined by building a terrain model of the lake and surrounding watersheds. Lake bottom elevations and land surface elevations were used to build the model with LP360 (by QCoherent) for ArcGIS, ESRI's ArcMap 10.4.1, the 3D Analyst ArcMap Extension, Python, and XTools Pro. The overall process involves merging the terrain morphology of the lake drainage basin with the underlying lake basin morphology to develop one continuous three-dimensional (3D) digital elevation model. The 3D digital elevation model was then used to calculate area of the lake and the associated volume of the lake at different elevations, starting at the

extent of the lake at its flood stage and working downward to the lowest elevation within the basin.

Table 1. Model inputs for the Pierce Lake water budget model.

Input Variable	Value
Overland Flow Watershed Size (acres)	114.9
SCS CN of watershed	68
Percent Directly Connected	0
FL Monitor Well(s) Used	Cross Bar 2SW CSX Floridan
Surf. Aq. Monitor Well(s) Used	Cross Bar 2SW CSX Surficial
Surf. Aq. Leakance Coefficient (ft/day/ft)	0.001
Fl. Aq. Leakance Coefficient (ft/day/ft)	0.001
Outflow 1 K	0.017
Outflow 1 Invert (ft NGVD29)	72.6
Outflow 2 K	0.010
Outflow 2 Invert (ft NGVD29)	73.4
Inflow K	N/A
Inflow Invert (ft NGVD29)	N/A

Precipitation

After a review of several rain gages in the area of Pierce Lake, the water budget model incorporated gage data plus NEXRAD (Next Generation Weather Radar), a network of 160 high-resolution Doppler weather radars controlled by the NWS, Air Force Weather Agency, and Federal Aviation Administration. The goal was to use the closest available data to the lake, as long as the data appeared to be high quality (Figure 9). The composite rainfall time series relied mainly on the Kent Grove gage (SID 20442; $n = 4,826$), located approximately 1.3 mi north-northwest of Pierce Lake, supported by NEXRAD (PIXEL 107909; $n = 2,845$) during times of missing or suspect gage data, namely most of 2000 to 2002 and 2017 to May 2018.

Lake Evaporation

Lake evaporation was estimated through use of monthly energy budget evaporation data collected by the U.S. Geological Survey (USGS) at Lake Starr in Polk County (Swancar and others, 2000) (Figure 10). The data was collected from August of 1996 through July of 2011. Monthly Lake Starr evaporation data were used in the Pierce Lake water budget model when available, and monthly averages for the period of record were used for those months when Lake Starr evaporation data were not available.

A recent study compared monthly energy budget evaporation data collected from both Lake Starr and Calm Lake (Swancar, 2011, personal communications). Calm Lake is

located approximately 13 miles south-southwest of Pierce Lake (Figure 10). The assessment concluded that the evaporation rates between the two lakes were nearly identical, with small differences attributed to measurement error and monthly differences in latent heat associated with differences in lake depth.

Jacobs (2007) produced daily potential evapotranspiration (PET) estimates on a 2-square kilometer grid for the entire state of Florida. The estimates begin in 1995, and are updated annually. These estimates, available from a website maintained by the USGS, were calculated using solar radiation data measured by a Geostationary Operational Environmental Satellite (GOES). Because PET is equal to lake evaporation over open water areas, using the values derived from the grid nodes over the modeled lake was considered. A decision was made to instead use the Lake Starr evaporation data since the GOES data nodes typically include both upland and lake estimates, with no clear way of subdividing the two. It was thought that using the daily PET estimates based on the GOES data would increase model error more than using the Lake Starr data directly.

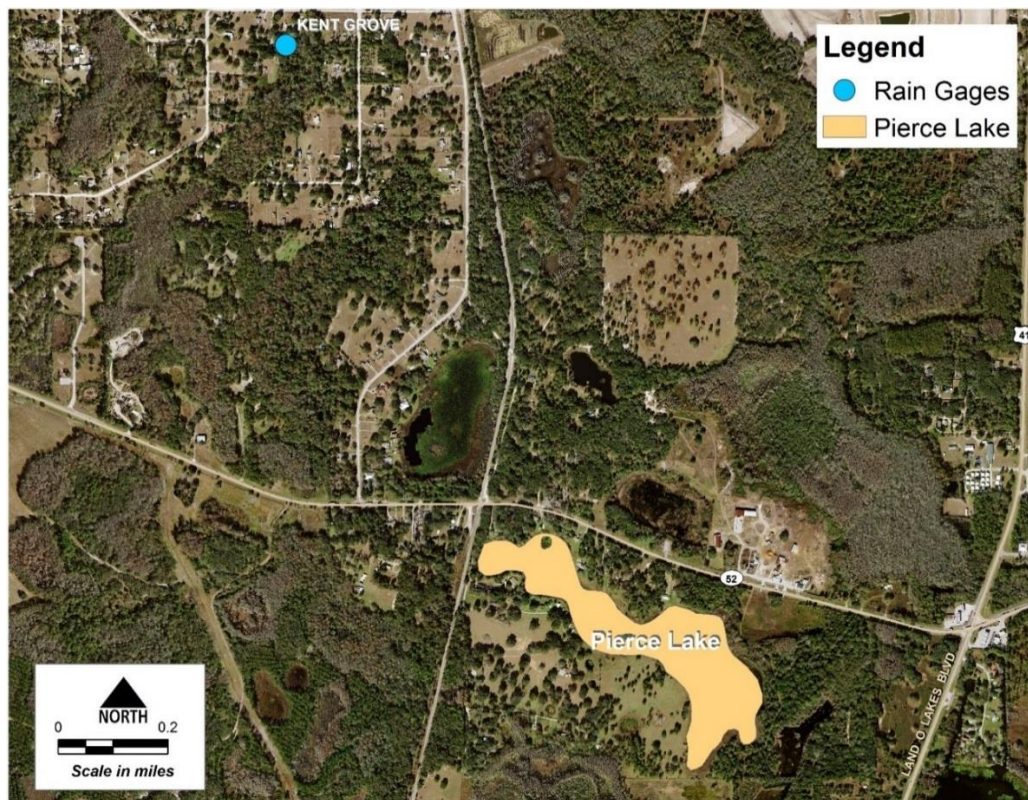


Figure 9. Rain gage (upper left) used in the Pierce Lake water budget model.

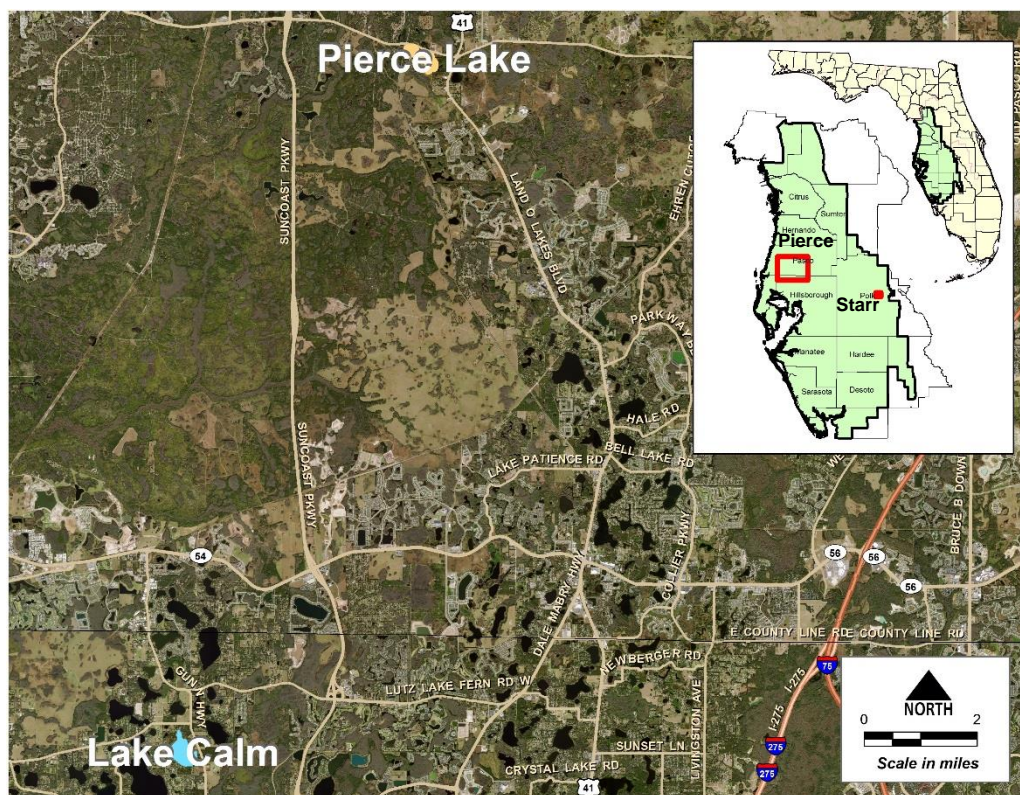


Figure 10. Location of Lakes Pierce, Calm and Starr (see map inset).

Jacobs (2007) produced daily potential evapotranspiration (PET) estimates on a 2-square kilometer grid for the entire state of Florida. The estimates begin in 1995, and are updated annually. These estimates, available from a website maintained by the USGS, were calculated using solar radiation data measured by a Geostationary Operational Environmental Satellite (GOES). Because PET is equal to lake evaporation over open water areas, using the values derived from the grid nodes over the modeled lake was considered. A decision was made to instead use the Lake Starr evaporation data since the GOES data nodes typically include both upland and lake estimates, with no clear way of subdividing the two. It was thought that using the daily PET estimates based on the GOES data would increase model error more than using the Lake Starr data directly.

Overland Flow

The water budget model was set up to estimate overland flow via a modified version of the U.S. Department of Agriculture, Soil Conservation Service (SCS) Curve Number method (SCS, 1972), and via directly connected impervious area calculations. The free water area of each lake was subtracted from the total watershed area at each time step to estimate the watershed area contributing to surface runoff. The directly connected impervious area (DCIA) was subtracted from the watershed for the SCS calculation, and

then added to the lake water budget separately. Additionally, the curve number (CN) chosen for the watershed of the lake considers the amount of DCIA in the watershed that has been handled separately.

The modified SCS method was suggested for use in Florida by CH2M HILL (2003), and has been used in several other analyses. The modification adds a fourth category of antecedent moisture condition (AMC) to the original SCS method (SCS, 1972) to account for Florida's frequent rainfall events.

The topography around Pierce Lake is relatively flat, so determining watersheds based on relatively subtle divides can be challenging. Several slightly varying estimates of watershed boundaries have been performed in the past for different modeling efforts in the area. The most recent set of estimates was developed as part of an effort to model the Bear Creek and Pithlachascotee River watersheds in Pasco County for flood assessment purposes (Singhofen and Associates, 2018). The watershed area values developed by Singhofen and Associates were adopted for the Pierce Lake model (Table 1) after an independent check confirming that they are reasonable for modeling purposes.

Pierce Lake's watershed as used in the model is shown in Figure 11. The entire area of the contributing watersheds is estimated to be approximately 114.9 acres (including the lake).

The DCIA and SCS CN used for the direct overland flow portion of the watershed are listed in Table 1. Curve numbers were difficult to assess. Most of the soils in the watershed are A/D soils (Adamsville and Smyrna fine sands), which means that the characteristics of the soils are highly dependent on how well they are drained. A "D" soil will generally have a higher amount of runoff per quantity of rain than a "A" soil. Because of the proximity of the wellfields to the area being modeled, water levels have been historically lowered by the withdrawals, and soils in the area may have had lower runoff rates (characteristic of "A" soils). Groundwater withdrawals during the latter period of model calibration were, however, significantly reduced relative to historic withdrawal rates, so the soils in the area may have begun to exhibit runoff properties more characteristic of "D" soils.

For purposes of this model, considering the range of conditions experienced, a CN was used somewhere between the two conditions. No direct discharges to the lake were identified, so the DCIA of the watershed is zero.

Inflow and Discharge via Channels from Outside Watersheds

Inflow and outflow via channels from or to the lake's watershed (i.e. "channel flow") can be important components of the Pierce Lake water budget, although the gradients of the

channels are relatively flat, and inflows to the lake likely occur only during very high rainfall events.

There is no channel inflow to Pierce Lake. To estimate flow out of Pierce Lake, the predicted elevation of the lake from the previous day is compared to the controlling elevation. Control elevations were determined based on professional surveying performed in the area. If the lake elevation is above the controlling elevation, the difference is multiplied by the current area of the lake and an “outflow coefficient.” The coefficient represents a measure of channel and structure efficiency, and produces a rough estimate of volume lost from the lake. This volume is then subtracted from the current estimate of volume in the lake.

Pierce Lake discharges via a ditch exiting the lake from its southern tip. The discharge passes across a field and enters a wetland to the south. The low point of the ditch, surveyed at 72.6 feet NGVD29, was determined to be the main controlling elevation for the outlet of Pierce Lake (Outflow 1, Table 1; pink line, Figure 2). Less frequently, Pierce Lake also discharges across a low spot in the pasture on its south-southwestern shore; the controlling elevation of this secondary discharge area was determined to be 73.4 feet NGVD29 (Outflow 2, Table 1; blue line, Figure 2).

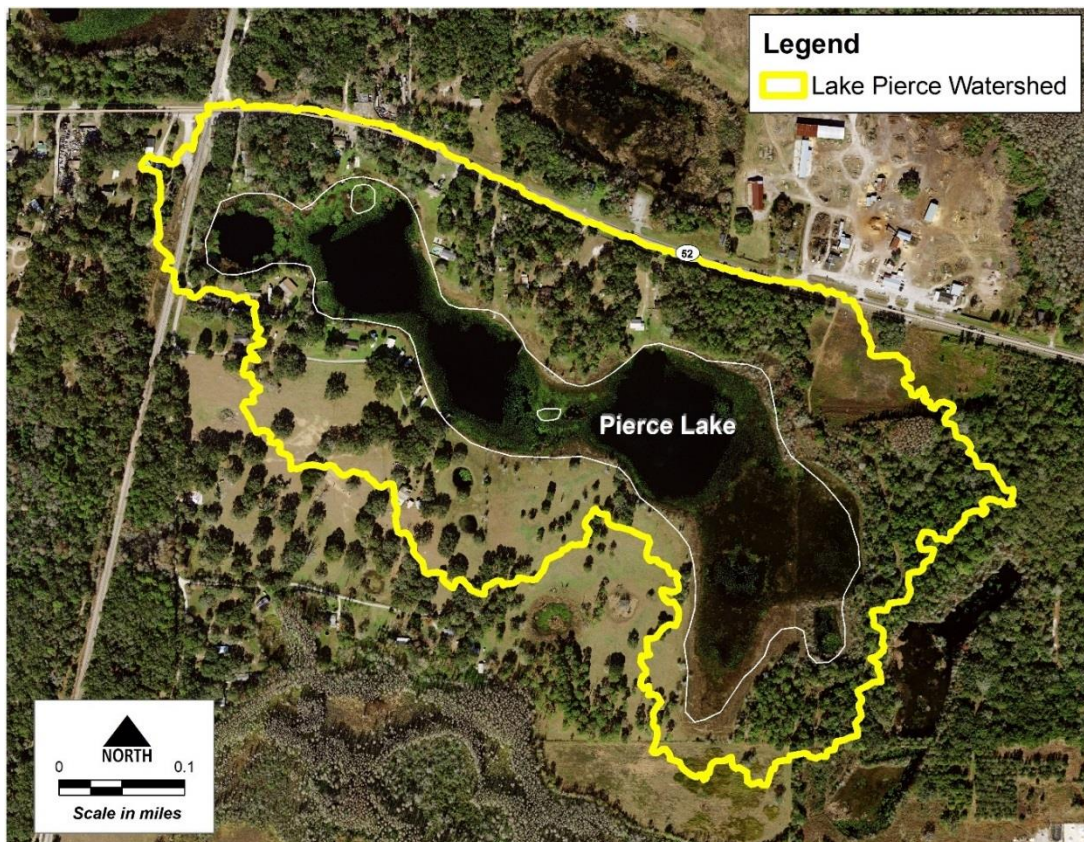


Figure 11. The Pierce Lake watershed (yellow line) used in the water budget model.

Flow from and into the surficial aquifer and Upper Floridan aquifer

Water exchange between Pierce Lake and underlying aquifers is estimated using a leakance coefficient and the head difference between the lake and the aquifer levels (Table 1). For each model time step, surficial aquifer and Upper Floridan aquifer leakage volumes were calculated independently. Leakance coefficients for each aquifer were determined through calibration.

Due to their proximity to Pierce Lake, one Upper Floridan monitor well (SID 20427) and one surficial aquifer monitor well (SID 20428) were considered for the analysis (Figure 4). The Cross Bar 2SW CSX Floridan and surficial aquifer monitor wells are each located approximately 0.3 miles from the lake's centroid and approximately 0.2 miles from the southwestern lakeshore. Data for these wells begin in May 1997 with daily data collection, with occasional gaps, occurring through the present (Figure 5). As discussed in "Physiography and Hydrogeology," confinement in the area of the Cross Bar 2SW CSX wells is generally low; from May 1997 to May 2018, surficial aquifer water level elevations at Cross Bar 2SW CSX exceeded those of the Upper Floridan by only 0.9 feet, on average, while correlating at $R^2 = 0.95$ (DF = 7,669). Other well nests in the area, such as Cross Bar 2E, display a similarly close relationship between the surficial and Upper Floridan aquifer. This suggests the the same adjustment can be reasonably applied to both the Upper Floridan and surficial aquifer levels.

Based on 2011 SWFWMD LiDAR (light detection and ranging) data, topographic elevations around Pierce Lake tend to vary around 73 ± 1.5 feet NGVD29, but the average elevation of land falling within 200 feet of the lake perimeter is approximately 73.9 feet NGVD29 (73.3 feet within 100 feet). The surficial well has a surveyed land surface elevation of 73.5 feet NGVD29, although the average land surface elevation within a 100 foot radius of the well is approximately 70.5 feet NGVD29 based on LiDAR data. Water level elevations at Cross Bar 2SW CSX Surficial average 3.3 feet lower than stage at Pierce Lake, similar to the 3.5 feet difference between elevations around the lake (73.9 feet NGVD29) and well (70.5 feet NGVD29). Additionally, inspection of multiple May and September potentiometric surface maps from District archives over several years representing periods before and after wellfield cutbacks found Pierce Lake located slightly upgradient of the well, with gradients of approximately 6 to 8 feet of head change for every 10,000 feet of horizontal distance (although this can vary significantly by season and year).

Based on the difference between average land surface elevations around the well and those around Pierce Lake, as well as typical head gradients in the Upper Floridan in the vicinity of Pierce Lake, water levels at both the Cross Bar 2SW CSX Upper Floridan and surficial wells were adjusted upwards by 1.5 feet, while capping maximum water levels at 73.9 feet NGVD29 (approximate ground elevation). The results were used to

represent the elevation of the Upper Floridan and surficial aquifer water levels under Pierce Lake. Missing data were infilled using bilinear interpolation.

F. Water Budget Model Approach

The primary reason for the development of the water budget model was to estimate Historic lake stage exceedance percentiles that could be used to support development of Minimum and Guidance Levels for the lake. Model calibration was therefore focused on matching long-term percentiles based on measured water levels, rather than short-term high and low levels.

Measured data from the lake were used for comparison with modeled water levels. Daily values are generated from the model, but only actual lake data points are used for the calibration.

Figure 13 presents the calibration results for the model. Table 2 presents a comparison of the percentiles of the measured data versus the model results. Table 3 presents modeled water budget components for the model calibration.

G. Water Budget Model Calibration Discussion

Based on a visual inspection of Figure 13, the model appears to be reasonably well calibrated. The model correlates with observed lake stage data at $R^2 = 0.94$ ($df = 244$). The mean and median differences of the residuals (observed less predicted values) are both -0.09 feet. The mean absolute error (MAE) is 0.39 feet, the root mean square error (RMSE) is 0.49 feet, and the Nash-Sutcliffe Efficiency (NSE) is 0.94.

A review of Table 2 shows that the differences for the P10 and P50 percentiles between the data and model for the lake are the same (within 0.1 feet), while the model P90 is 0.3 feet higher.

There are a few periods when the peaks in the modeled hydrograph are higher or lower than the measured values, and these differences contributed to minor differences between the modeled and measured percentiles associated with higher and lower lake levels, i.e., the P10 and P90 percentiles. The minimum and maximum differences are -1.93 and 1.66 feet, respectively. Reduced precision in the higher and lower ranges of the stage-volume relationships for the lake may also have contributed to the percentile differences.

The water budget component values in the model can be difficult to judge since they are expressed as inches per year over the average lake area for the period of the model run. Leakage rates (and leakage coefficients), for example, represent conditions below the lake only, and may be very different than those values expected in the

general area. Runoff also represents a volume over the average lake area, and when the resulting values are divided by the watershed area, they represent low runoff rates.

Table 2. Comparison of percentiles of measured lake level data compared to calibration percentiles from the model (all in feet NGVD29).

	Data	Model
P10	72.6	72.6
P50	71.0	71.0
P90	67.1	67.4

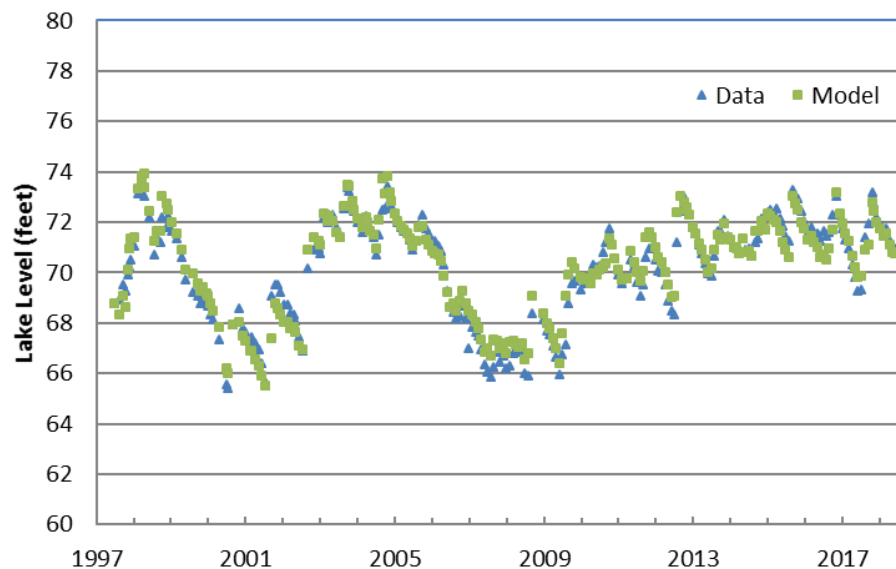


Figure 13. Modeled water levels predicted for the calibrated Pierce Lake water budget model (Model; green squares) and measured levels used for the model calibration (Data; blue triangles).

Table 3. Pierce Lake Water Budget (May 1997 to May 2018).

Inflows	Rainfall	Surficial Aquifer Groundwater Inflow	Floridan Aquifer Groundwater Inflow	Runoff	DCIA Runoff	Inflow via channel	Total
Inches/year	57.2	0.1	0.0	25.9	0.0	0.0	83.3
Percentage	68.7	0.1	0.0	31.2	0.0	0.0	100.0
Outflows	Evaporation	Surficial Aquifer Groundwater Outflow	Floridan Aquifer Groundwater Outflow			Outflow via channel	Total
Inches/year	58.1	8.1	11.8			4.4	82.4
Percentage	70.5	9.9	14.3			5.4	100.0

H. Water Budget Model Results

Groundwater withdrawals are not directly included in the Pierce Lake water budget model, but are indirectly represented by their effects on water levels in the Upper Floridan aquifer. Metered groundwater withdrawal rates from Cross Bar Ranch and North Pasco wellfields are available throughout the period of the calibrated model, so if a relationship between withdrawal rates and Upper Floridan aquifer potentiometric levels can be established, the effect of changes in groundwater withdrawals can be estimated by adjusting Upper Floridan aquifer levels in the model.

The Integrated Northern Tampa Bay (INTB) model (Geurink and Basso, 2013) is an integrated model developed for the northern Tampa Bay area. The INTB model can account for groundwater and surface-water, as well as the interaction between them. The domain of the INTB application includes the Pierce Lake area, and represents the most current understanding of the hydrogeologic system in the area.

The INTB was used to determine the drawdown in the surficial aquifer and Upper Floridan aquifer in response to groundwater withdrawals in the area. Drawdown in both aquifers was calculated for two withdrawal rates representing the effects of Tampa Bay Water's regional wellfields before and after cutbacks from approximately 150 mgd to 90 mgd. The pre-cutback period in the model is from May 1997 through 2002, while the post-cutback period is 2003 through May 2018. (Although the North Pasco Wellfield did not experience cutbacks until 2008, as shown in Figure 7, its contribution to regional drawdown in the vicinity of Pierce Lake was overshadowed by the relatively larger pumping at Cross Bar Ranch Wellfield.) The model results allowed the drawdowns associated with all permitted withdrawals to be calculated before and after wellfield cutbacks, assuming changes in all other withdrawals are consistent for the modeled period.

The INTB model was run for each withdrawal scenario from 1996 to 2006 using a daily integration step. Drawdown values in feet were calculated by running the model with and without groundwater withdrawals, and were calculated for each node in the model. The INTB model uses a one-quarter mile grid spacing around the wellfields. Groundwater withdrawal rates from the Cross Bar Ranch Wellfield in each scenario were 22.5 mgd and 16.0 mgd, respectively, and 1.6 mgd and 0.3 mgd for the North Pasco Wellfield, respectively.

Results from the INTB modeling scenarios showed that there is a fairly linear relationship between Upper Floridan aquifer drawdown and withdrawal rates at the wellfields. Because of the leaky nature of the confining unit around Pierce Lake, and because the water table in the model is not active, the relationship between

groundwater withdrawals in the Upper Floridan and water levels in the surficial aquifer was also of interest. Using the drawdowns determined through the INTB model, the Upper Floridan aquifer and surficial monitor well data in the model can be adjusted to reflect changes in groundwater withdrawals.

To estimate lake levels without the influence of groundwater withdrawals, the Upper Floridan aquifer and surficial aquifer wells in the water budget model were adjusted to represent zero withdrawals. For the May 1997 through May 2018 water budget model period, two adjustment periods were used to reflect the cutbacks that took place at the Cross Bar Ranch Wellfield. Adjustments to each Upper Floridan aquifer and surficial aquifer well and the associated adjustment periods are found in Table 4.

Figure 14 presents measured water level data for the lake along with the model-simulated lake levels in the lake under Historic conditions, i.e. in the absence of groundwater withdrawals with structural alterations similar to current conditions. Table 5 presents the Historic percentiles based on the model output.

Table 4. Aquifer water level adjustments to the Pierce Lake Model to represent Historic percentiles.

Well	Adjustment (feet) May 1997 through 2002	Adjustment (feet) 2003 through May 2018
Floridan aquifer	0.5	0.2
Surficial aquifer	0.5	0.2

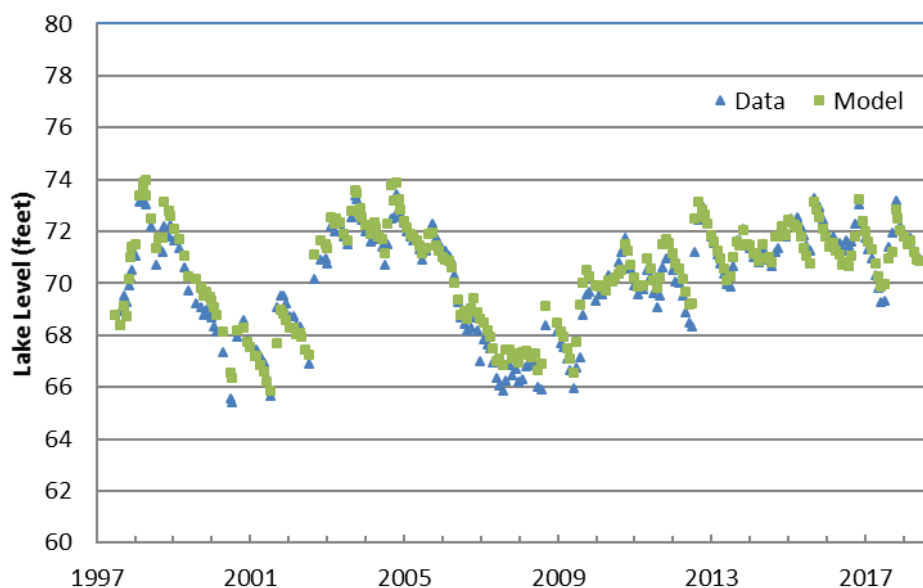


Figure 14. Measured lake levels and Historic water levels predicted with the calibrated Pierce Lake model.

Table 5. Historic percentiles estimated using the Pierce Lake water budget model (in feet NGVD29).

Percentile	Elevation
P10	72.7
P50	71.1
P90	67.5

I. Rainfall Correlation Model

A line of organic correlation (LOC) was performed using the results of the water budget model and long-term rainfall to extend the data set used to determine the Historic percentiles. These Historic percentiles are considered in development of the Minimum Levels. The LOC is a linear fitting procedure that minimizes errors in both the x and y directions and defines the best-fit straight line as the line that minimizes the sum of the areas of right triangles formed by horizontal and vertical lines extending from observations to the fitted line (Helsel and Hirsch, 2002). LOC is preferable for this application since it produces a result that best retains the variance (and therefore best retains the "character") of the original data.

In this application, the simulated lake water levels representing Historic conditions were correlated with Long-term rainfall. For the correlation, additional representative rainfall records were added to the rainfall records used in the water budget model (May 1997 to May 2018). Rainfall data from the Kent Grove gage (SID 20442; 1.3 miles north-northwest of Pierce Lake) was used to extended data back for 1986, followed by the Gowners Corner Tower gage (SID 20189; 2.2 miles west of Pierce Lake) from 1986 to 1984, the Gowners Corner gage (SID 20188; 0.6 miles east of Pierce Lake) from 1984 to 1973, the Myrtle Lake gage (SID 19057; 9.8 miles south-southeast of Pierce Lake) from 1973 to 1967, the Lutz gage (SID 19629; 11.1 miles south of Pierce Lake) from 1967 to 1963, and the Saint Leo NWS gage (SID 18901; 15.4 miles east of Pierce Lake) from 1962 to 1930. The Brooksville Chinsegut Hill (SID 20573; 22.4 miles north-northeast of Pierce Lake; $n = 40$) and Crews Lake West (SID 20536; 6.5 miles north of Pierce Lake; $n = 30$) gages provided minor support during data gaps.

Rainfall is correlated to lake water level data by applying a linear inverse weighted sum to the rainfall. The weighted sum gives higher weight to more recent rainfall and less weight to rainfall in the past. In this application, weighted sums varying from 6 months to 10 years are separately used, the results are compared, and the correlation with the highest correlation coefficient (R^2) is chosen as the best model.

Rainfall was correlated to the water budget model results for the entire period used in the water budget model (May 1997 to May 2018), and the results from January 1946 to

May 2018 (~71.4 years) were produced. For Pierce Lake, the 2-year weighted model had the highest correlation coefficient, with an R^2 of 0.88 (the 3-year was a close second with an R^2 of 0.87 but resulted in nearly identical percentiles, except the P50 and P90 were each 0.1 feet higher). Previous correlations for lakes in the northern Tampa Bay area have consistently had best correlation coefficients in the 2 to 5-year range. The results are presented in Figure 16.

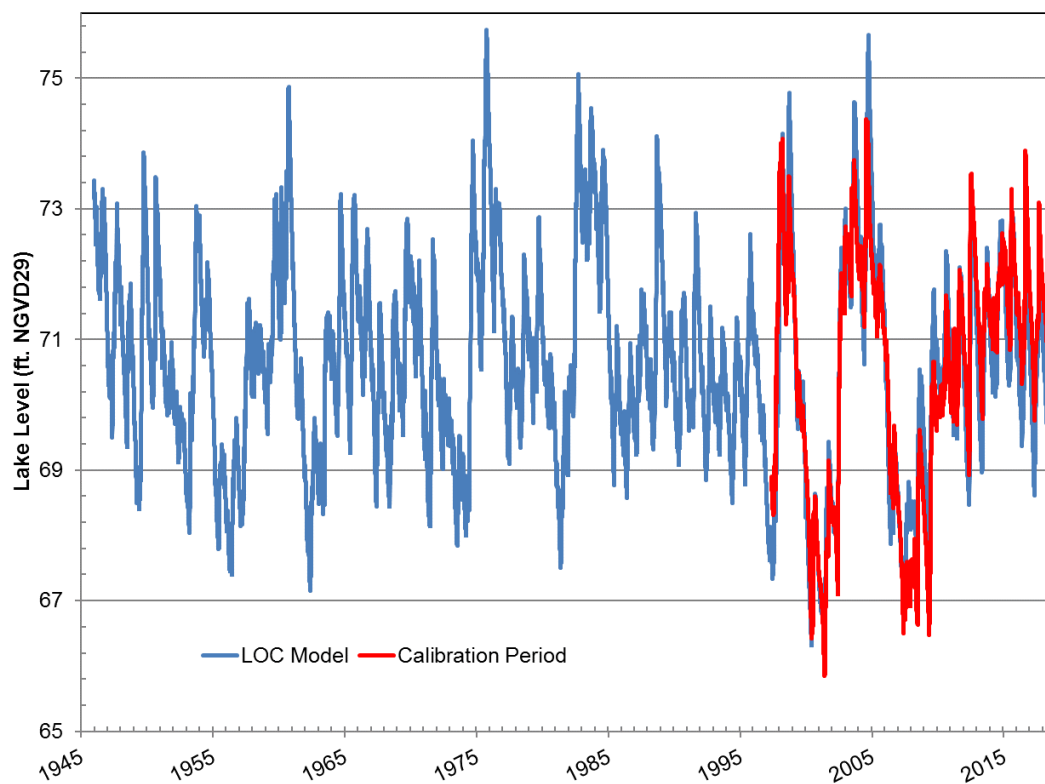
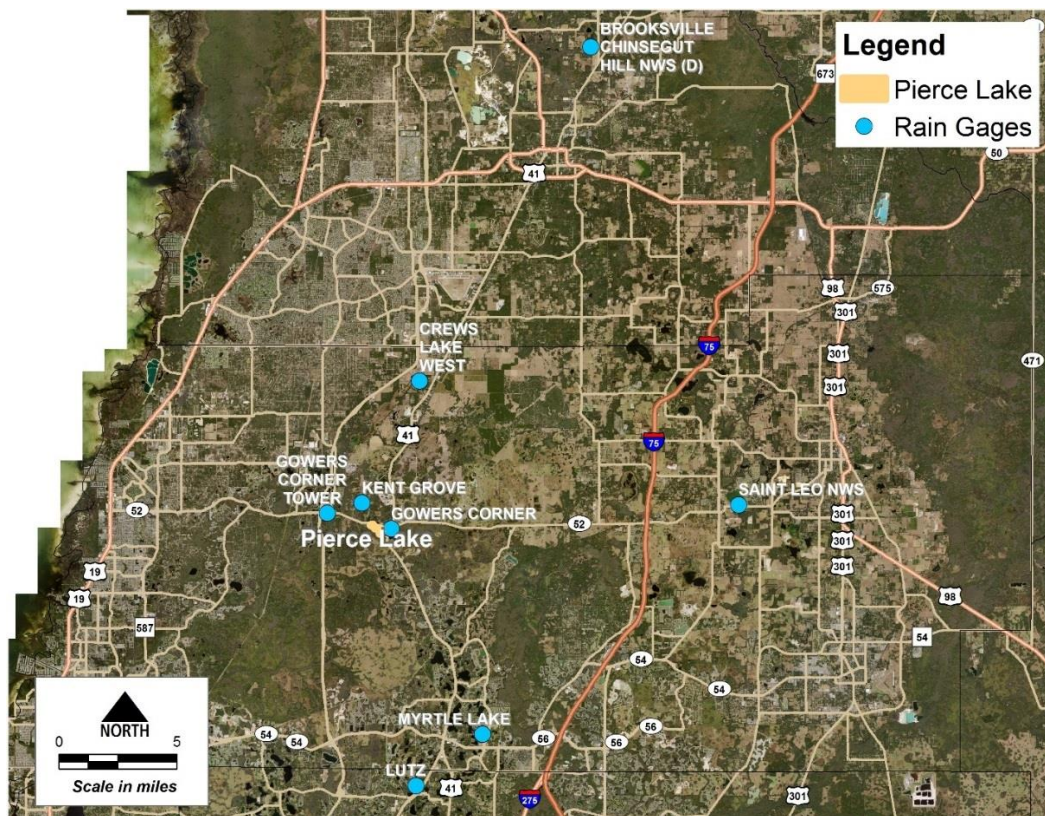
To produce Historic percentiles that apply significant weight to the results of the water budget models, the rainfall LOC results for the period of the water budget model are replaced with the water budget model results. Therefore, the LOC rainfall model results are used for the period of 1946 through April 1997, while the water budget results are used for the period of May 1997 through May 2018. These results are referred to as the “hybrid model.” The resulting Historic percentiles for the hybrid model are presented in Table 6. Note that the the P10, P50, and P90 percentiles for the water budget model (Table 5) differ from those of the hybrid rainfall model (Table 6) for Pierce Lake by 0.0, 0.5, and 1.2 feet, respectively.

Table 6. Historic percentiles as estimated by the hybrid model from January 1946 to May 2018 (feet NGVD29).

Percentile	Pierce Lake
P10	72.7
P50	70.6
P90	68.5

J. Conclusions

Based on the model results and the available data, the Pierce Lake water budget and LOC rainfall models are useful tools for assessing long-term percentiles in the lake. Based on the same information, lake stage exceedance percentiles developed through use of the models appear to be reasonable estimates for Historic conditions.



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APPENDIX B

Technical Memorandum

August 31, 2018

TO: Tamera S. McBride, P.G., Manager, Resource Evaluation, Water Resources Bureau

FROM: Cortney Cameron, G.I.T., Hydrogeologist, Water Resources Bureau
Michael C. Hancock, P.E., Chief Prof. Engineer, Water Resources Bureau

Subject: Pierce Lake Initial Minimum Levels Status Assessment

A. Introduction

The Southwest Florida Water Management District (District) is reevaluating adopted minimum levels for Pierce Lake and is proposing revised minimum levels for the lake, in accordance with Section 373.042 and 373.0421, Florida Statutes (F.S.). Documentation regarding development of the revised minimum levels is provided by Cameron and Hancock (2018) and Carr and others (2018).

Section 373.0421, F.S. requires that a recovery or prevention strategy be developed for all water bodies that are found to be below their minimum flows or levels, or are projected to fall below the minimum flows or levels within 20 years. In the case of Pierce Lake and other waterbodies with established minimum flows or levels in the northern Tampa Bay area, an applicable regional recovery strategy, referred to as the “Comprehensive Plan”, has been developed and adopted into District rules (Rule 40D-80.073, F.A.C.). One of the goals of the Comprehensive Plan is to achieve recovery of minimum flow and level water bodies such as Pierce Lake that are in the area affected by the Consolidated Permit wellfields (i.e., the Central System Facilities) operated by Tampa Bay Water. This document provides information and analyses to be considered for evaluating the status (i.e., compliance) of the revised minimum levels proposed for Pierce Lake and any recovery that may be necessary for the lake.

B. Background

Pierce Lake is in west-central Pasco County, southwest of the intersection between State Road 52 and U.S. Highway 41 (Figure 1). The lake lies within the Gowers Corner Slough basin that forms part of the larger Crystal-Pithlachascotee (also called the Upper Coastal) watershed (USGS HUC 03100207). Pierce Lake has no significant inflow other than overland flow. At stages above approximately 72.6 ft NGVD29, Pierce Lake discharges through a ditch

at its southern end across a field; additionally, at stages above approximately 73.4 ft NGVD29, Pierce Lake also discharges across a low spot in a pasture along its south-southwestern shore (Figure 2). Both outflows enter the same wetland to the lake's southwest. The topography is very flat, however, and flows are often negligible.

Pierce Lake is located south-southwest of Cross Bar Ranch Wellfield, one of ten regional water supply wellfields operated by Tampa Bay Water (Figure 6). North Pasco Wellfield, to the west of Pierce Lake, discontinued operation in 2018. While Pierce Lake is located several miles away from the nearest pumping well (North Pasco included), it nevertheless experienced some drawdown as a result of groundwater withdrawals (Appendix C). At the Cross Bar Ranch Wellfield, groundwater withdrawals began in 1980 at a rate of approximately 7 gallons per day (mgd), steadily climbing to a peak of 41 mgd by the end of 1993, after which it declined until cutbacks occurred in 2003, averaging 15 mgd thereafter. North Pasco Wellfield began operation in 1992, averaging 2 mgd until cutbacks in 2008, after which it averaged under 1 mgd until its closure in 2018.

C. Revised Minimum Levels Proposed for Pierce Lake

Revised minimum levels proposed for Pierce Lake are presented in Table 1 and discussed in more detail by Carr and others (2018). Minimum levels represent long-term conditions that, if achieved, are expected to protect water resources and the ecology of the area from significant harm that may result from water withdrawals. The Minimum Lake Level is the elevation that a lake's water levels are required to equal or exceed fifty percent of the time on a long-term basis. The High Minimum Lake Level is the elevation that a lake's water levels are required to equal or exceed ten percent of the time on a long-term basis. The Minimum Lake Level therefore represents the required 50th percentile (P50) of long-term water levels, while the High Minimum Lake Level represents the required 10th percentile (P10) of long-term water levels. To determine the status of minimum levels for Pierce Lake or minimum flows and levels for any other water body, long-term data or model results must be used.

Table 1. Proposed Minimum Levels for Pierce Lake.

Proposed Minimum Levels	Elevation in Feet NGVD 29
High Minimum Lake Level	71.9
Minimum Lake Level	69.8

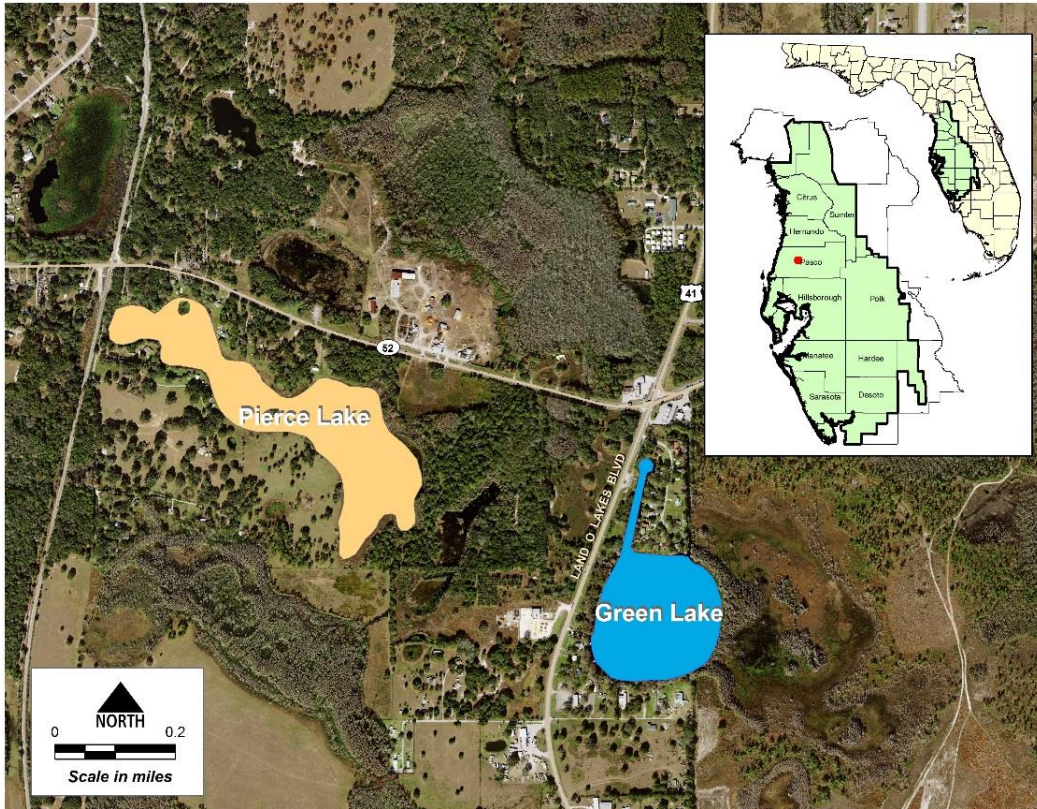


Figure 1. Location of Pierce Lake in Pasco County, Florida.

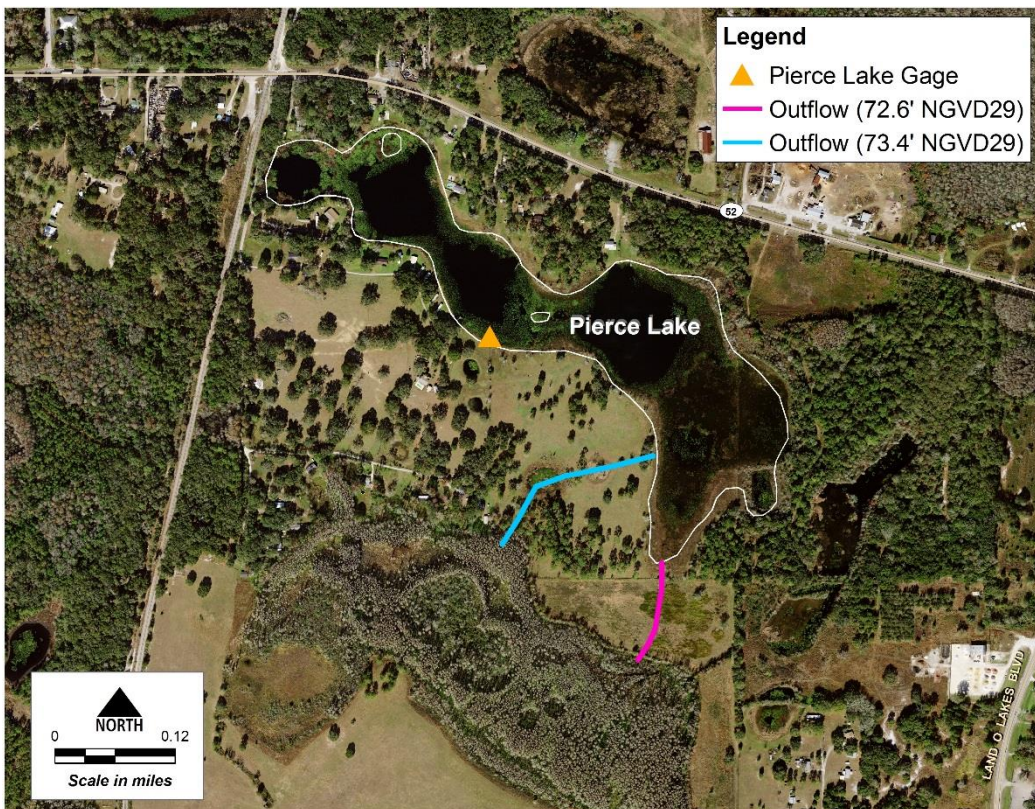


Figure 2. Flow out of Pierce Lake (pink and blue lines).

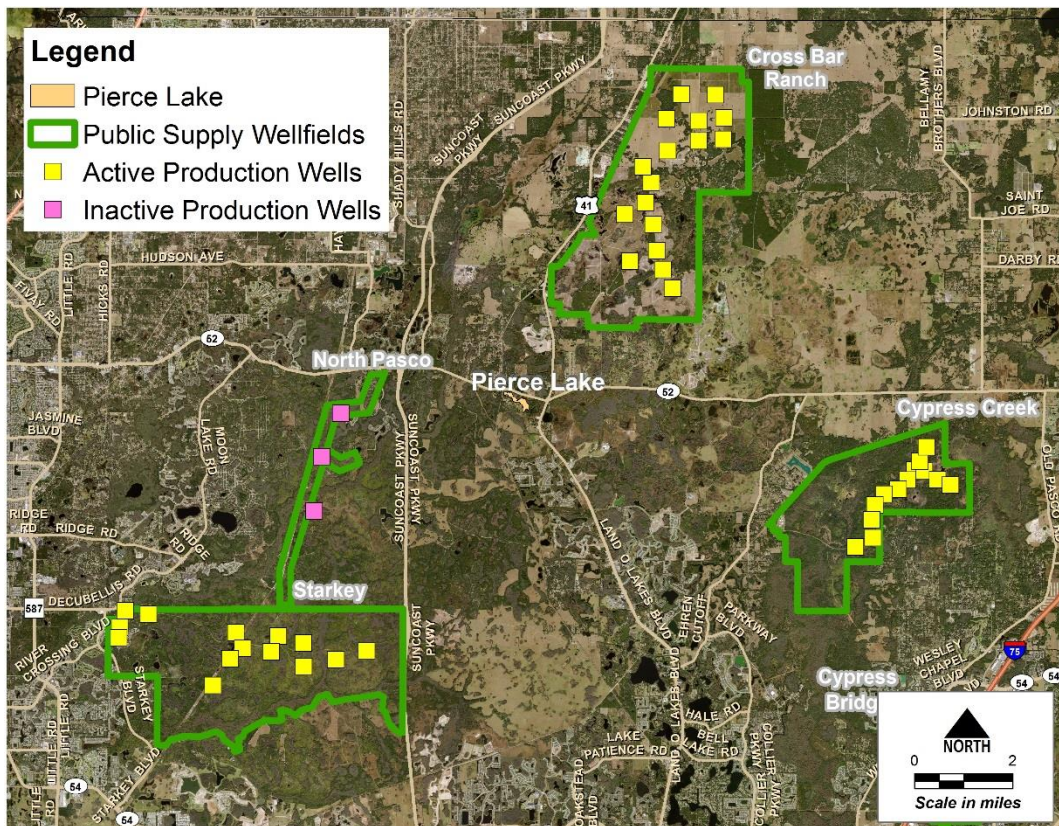


Figure 3. Pierce Lake (center) and (clockwise from Pierce) the Cross Bar Ranch, Cypress Creek, Starkey, and North Pasco wellfields.

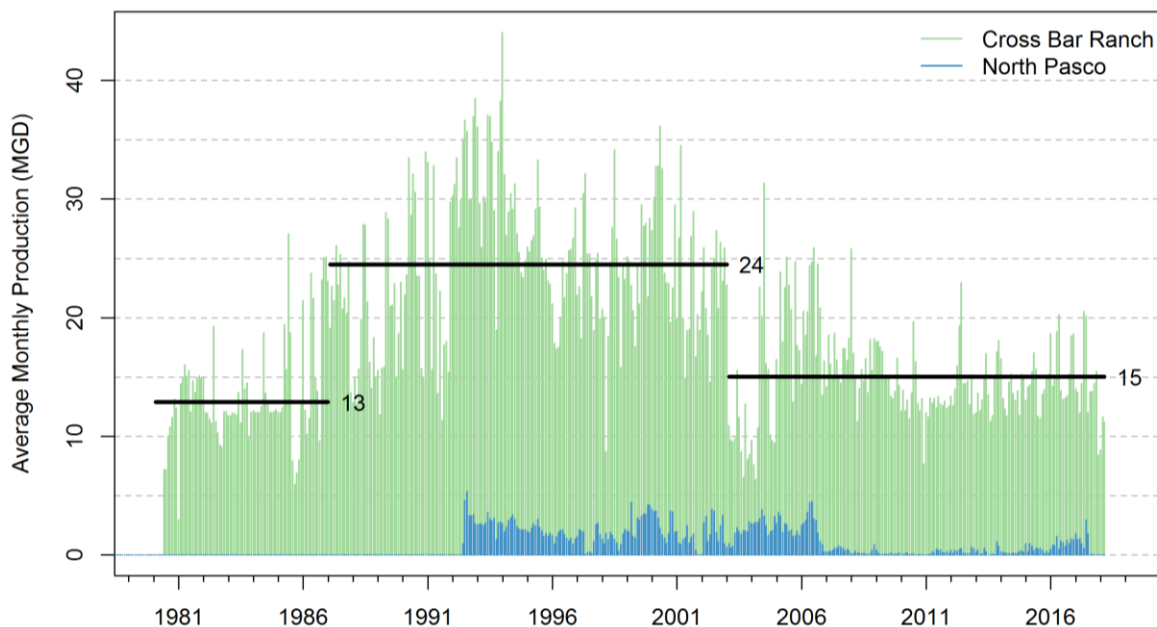


Figure 4. Stacked Cross Bar Ranch (red) and North Pasco (green) Wellfield average monthly withdrawals. Horizontal black lines indicate the average combined production (mgd) during the time period spanned by the line.

D. Status Assessment

The lake status assessment approach involves using actual lake stage data for Pierce Lake from January 2003 through May 2018, which was determined to represent the “Current” period based on the commencement of cutbacks at the Cross Bar Ranch Wellfield (Figure 4). The Current period represents a recent “Long-term” period when hydrologic stresses (including groundwater withdrawals) and structural alterations are reasonably stable. “Long-term” is defined as a period that has been subjected to the full range of rainfall variability that can be expected in the future.

As demonstrated in Cameron and Hancock (2018), groundwater withdrawals during this period were relatively consistent (Figure 4). To create a data set that can reasonably be considered “Long-term,” a regression analysis using the line of organic correlation (LOC) method was performed on the lake level data from the Current period. The LOC is a linear fitting procedure that minimizes errors in both the *x* and *y* directions and defines the best-fit straight line as the line that minimizes the sum of the areas of right triangles formed by horizontal and vertical lines extending from observations to the fitted line (Helsel and Hirsch, 2002). The LOC is preferable for this application since it produces a result that best retains the variance (and therefore best retains the “character”) of the original data. This technique was used to develop the minimum levels for Pierce Lake (Cameron and Hancock, 2018). By using this technique, the limited years of Current lake level data can be projected back to create a simulated data set representing over 70 years of lake levels, based on the current relationship between lake water levels and actual rainfall.

The same rainfall data set used for setting the minimum levels for Pierce Lake was used for the status assessment (Cameron and Hancock, 2018). The best resulting correlation for the LOC model created with measured data (January 2003 to May 2018) was the 3-year weighted period, with a coefficient of determination of 0.81. The resulting lake stage exceedance percentiles are presented in Table 2.

As an additional piece of information, Table 2 also presents the percentiles calculated directly from the measured lake level data for Pierce Lake for the period from January 2003 through May 2018. A limitation of these values is that the resulting lake stage exceedance percentiles are representative of rainfall conditions during only the past 13 years, rather than the longer-term rainfall conditions represented in the January 1946 to May 2018 LOC model simulation.

A comparison of the LOC model with the revised minimum levels proposed for Pierce Lake indicates that the Long-term P10 is 0.7 feet above the proposed High Minimum Lake Level, and the Long-term P50 is 0.5 feet above the proposed Minimum Lake Level. The P10 elevation derived directly from the January 2003 to May 2018 measured lake data is 0.4 feet above the proposed High Minimum Lake Level, and the P50 elevation is 1.5 feet above the proposed Minimum Lake Level. Differences in rainfall between the shorter January 2003 to

May 2018 period and the longer 1946 to 2017 period used for the LOC modeling analyses likely contribute to the differences between derived and measured lake stage exceedance percentiles. Additionally, differences between actual withdrawal rates and those used in the models may have contributed to some of the differences in the percentiles.

Table 2. Comparison of lake stage exceedance percentiles derived from the lake stage/LOC results, exceedance percentiles of the January 2003 to May 2018 data, and the revised minimum levels proposed for Pierce Lake. All elevations in feet NGVD 29.

Percentile	Proposed Minimum Levels	Long Term LOC Model Results (1946 to 2018*)[†]	Measured Lake Levels for Current Period (2003 to 2018*)	Mean Monthly Measured Lake Levels for Current Period (2003 to 2018*)[‡]
P10	71.9	72.6	72.3	72.5
P50	69.8	70.3	71.3	71.2

* May 2018

[†] LOC model based on Current Period and extended using rainfall for 1946 to May 2018.

[‡] Stages aggregated to the monthly level using monthly mean to reduce biasing from varying data collection frequency.

E. Conclusions

Based on the information presented in this memorandum, it is concluded that Pierce Lake water levels are above the revised Minimum Lake Level and above the revised High Minimum Lake Level proposed for the lake. These conclusions are supported by comparison of percentiles derived from Long-term LOC modeled lake stage data with the proposed minimum levels.

Minimum flow and level status assessments are completed on an annual basis by the District and on a five-year basis as part of the regional water supply planning process. In addition, Pierce Lake are included in the Comprehensive Environmental Resources Recovery Plan for the Northern Tampa Bay Water Use Caution Area (40D-80.073, F.A.C). Therefore, the status of Pierce Lake will be reassessed by the District and Tampa Bay Water as part of this plan, and as part of Tampa Bay Water's Permit Recovery Assessment Plan (required by Chapter 40D-80, F.A.C. and the Consolidated Permit (No. 20011771.001)). Tampa Bay Water, in cooperation with the District, will assess the specific needs for recovery in Pierce Lake and other water bodies affected by groundwater withdrawals from the Central System Facilities. By 2020, if not sooner, an alternative recovery project will be proposed if Pierce Lake are found to not be meeting their adopted minimum levels. The draft results of the Permit Recovery Assessment Plan are due to the District by December 31, 2018.

F. References

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APPENDIX C

Technical Memorandum

September 4, 2018

TO: David Carr, Environmental Scientist, Water Resources Bureau
Michael C. Hancock, P.E., Chief Prof. Engineer, Water Resources Bureau
Cortney Cameron, G.I.T., Hydrogeologist, Water Resources Bureau

FROM: Cortney Cameron, G.I.T., Hydrogeologist, Water Resources Bureau

Subject: Evaluation of Groundwater Withdrawal Impacts to Pierce Lake

A. Introduction

Pierce Lake is located in central Pasco County in west-central Florida (Figure 1). Prior to establishment of a Minimum Level (ML), an evaluation of hydrologic changes in the vicinity of the lake is necessary to determine if the water body has been significantly impacted by groundwater withdrawals. The establishment of the ML for Pierce Lake is not part of this report. This memorandum describes the hydrogeologic setting near the lake and includes the results of two numerical model scenarios of groundwater withdrawals in the area.

B. Hydrogeologic Setting

The hydrogeology of the area includes a surficial sand aquifer system; a discontinuous, intermediate clay confining unit, a thick carbonate Upper Floridan aquifer, a low permeable confining unit and a Lower Floridan aquifer. In general, the surficial aquifer system is in good hydraulic connection with the underlying Upper Floridan aquifer because the clay confining unit is generally thin, discontinuous, and breached by numerous karst features. The surficial sand aquifer is generally a few tens of feet thick and overlies the limestone of the Upper Floridan aquifer that averages nearly 1,000 feet thick in the area (Miller, 1986). In between these two aquifers is the Hawthorn Group clay that varies between a few feet to as much as 25 feet thick. Because the clay unit is breached by buried karst features and has previously been exposed to erosional processes, preferential pathways locally connect the overlying surficial aquifer to the Upper Floridan aquifer resulting in moderate-to-high leakage to the Upper Floridan aquifer (SWFWMD, 1996). Thus, the Upper Floridan aquifer is defined as a leaky artesian aquifer system.

The base of the Upper Floridan aquifer generally occurs at the first, persistent sequence of evaporitic minerals such as gypsum or anhydrite that occur as nodules or discontinuous thin layers in the carbonate matrix. This low permeability unit is regionally extensive and is generally referred to as middle confining unit II. Underlying the middle confining unit II is the Lower Floridan aquifer (Miller, 1986).

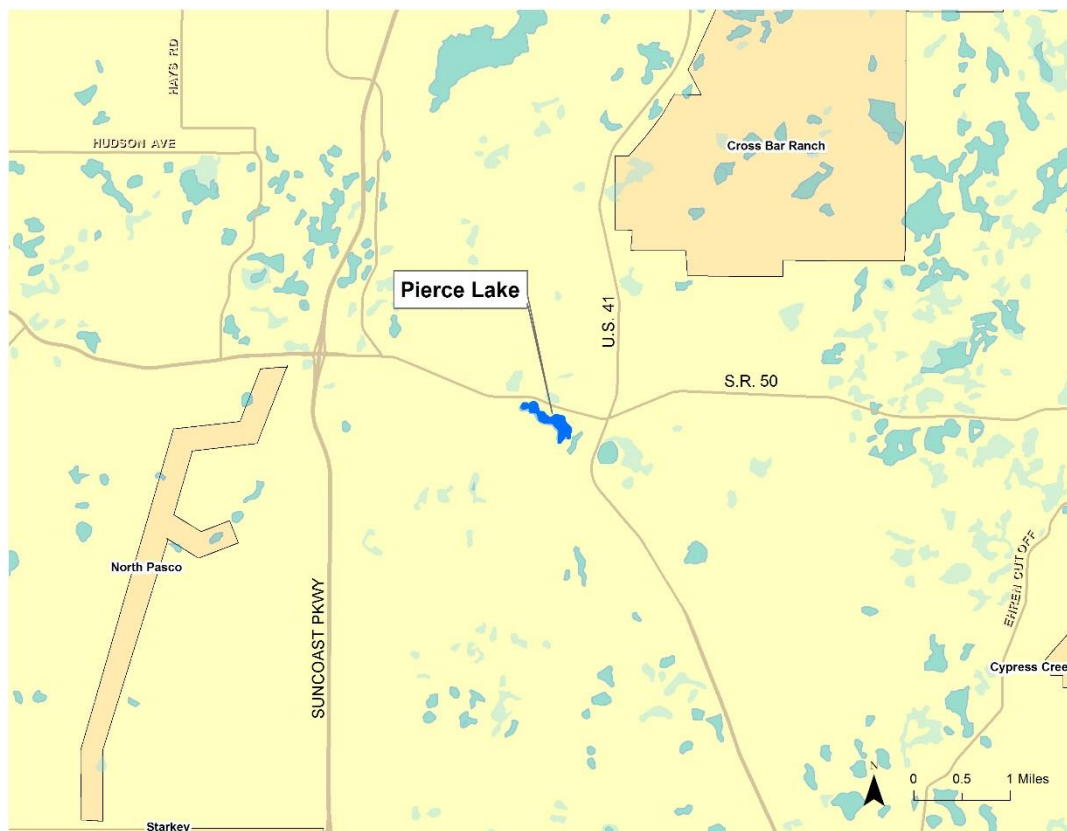


Figure 1. Location of Pierce Lake.

C. Evaluation of Groundwater Withdrawal Impacts to Pierce Lake

Several regional groundwater flow models have included the area around Pierce Lake in central Pasco. Ryder (1982) simulated the entire extent of the Southwest Florida Water Management District. In 1993, the District completed the Northern Tampa Bay groundwater flow model that covered a 2,000-square mile area of Hillsborough, Pinellas, Pasco, and Hernando Counties (SWFWMD, 1993). In 2002, the USGS simulated the entire Florida peninsula in their Mega Model of regional groundwater flow (Sepulveda, 2002). The most recent and advanced simulation of southern Pasco County and the surrounding area is the Integrated Northern Tampa Bay (INTB) model (Geurink and Basso, 2012). The construction and calibration of this model was part of a cooperative effort between the SWFWMD and Tampa Bay Water (TBW), a regional water utility that operates 11 major wellfields. The Integrated Northern Tampa Bay Model covers a 4,000 square-mile area of the Northern Tampa Bay region (Figure 2).

An integrated model represents the most advanced simulation tool available to the scientific community in water resources investigations. It combines the traditional ground-water flow model with a surface water model and contains an interprocessor code that links both systems. One of the many advantages of an integrated model is that it simulates the entire hydrologic system. It represents the “state-of-art” tool in assessing changes due to rainfall, drainage alterations, and withdrawals.

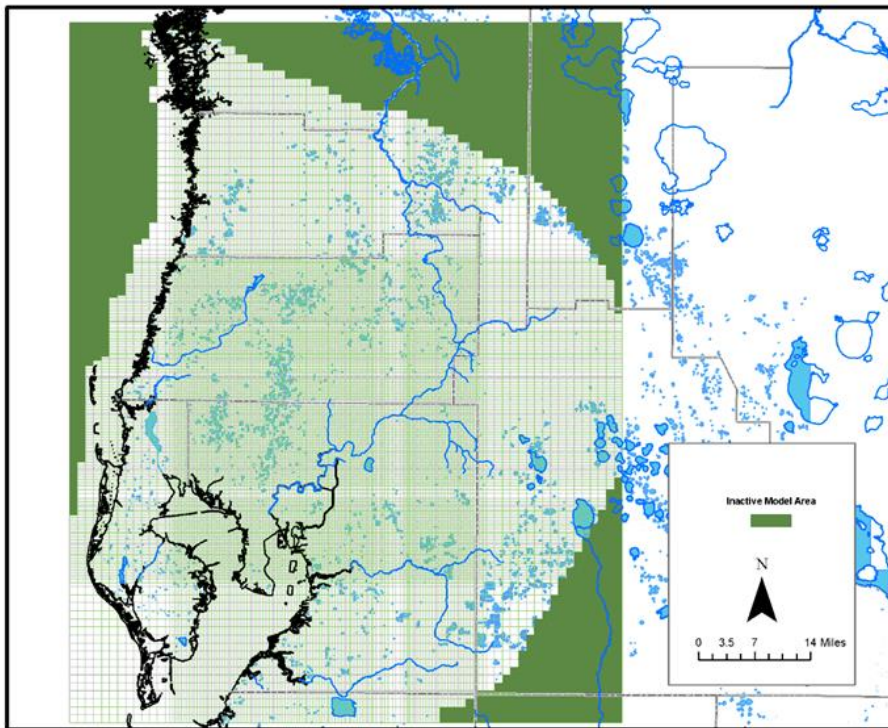


Figure 2. Groundwater grid used in the INTB model.

The model code used to run the INTB simulation is called the Integrated Hydrologic Model (IHM) which combines the HSPF surface water code and the MODFLOW ground-water code using interprocessor software. During the INTB development phase, several new enhancements were made to move the code toward a more physically-based simulation. The most important of these enhancements was the partitioning of the surface into seven major land use segments: urban, irrigated land, grass/pasture, forested, open water, wetlands, and mining/other. For each land segment, parameters were applied in the HSPF model consistent with the land cover, depth-to-water table, and slope. Recharge and ET potential were then passed to each underlying MODFLOW grid cell based on an area weighted-average of land segment processes above it. Other new software improvements included a new ET algorithm/hierarchy plus allowing the model code to transiently vary specific yield and vadose zone storages.

The INTB model contains 172 subbasin delineations in HSPF (Figure 3). There is also an extensive data input time series of 15-minute rainfall from 300 stations for the period 1989-1998, a well pumping database that is independent of integration time step (1-7 days), a methodology to incorporate irrigation flux into the model simulation, construction of an approximate 150,000 river cell package that allows simulation of hydrography from major rivers to small isolated wetlands, and GIS-based definition of land cover/topography. An empirical estimation of ET was also developed to constrain model derived ET based on land use and depth-to-water table relationships.

The MODFLOW gridded domain of the INTB contains 207 rows by 183 columns of variable spacing ranging from 0.25 to one mile. The groundwater portion is comprised of three layers: a surficial aquifer (layer 1), an intermediate confining unit or aquifer (layer 2), and the Upper Floridan aquifer (layer 3). The model simulates leakage between layers in a quasi-3D manner through a leakance coefficient term.

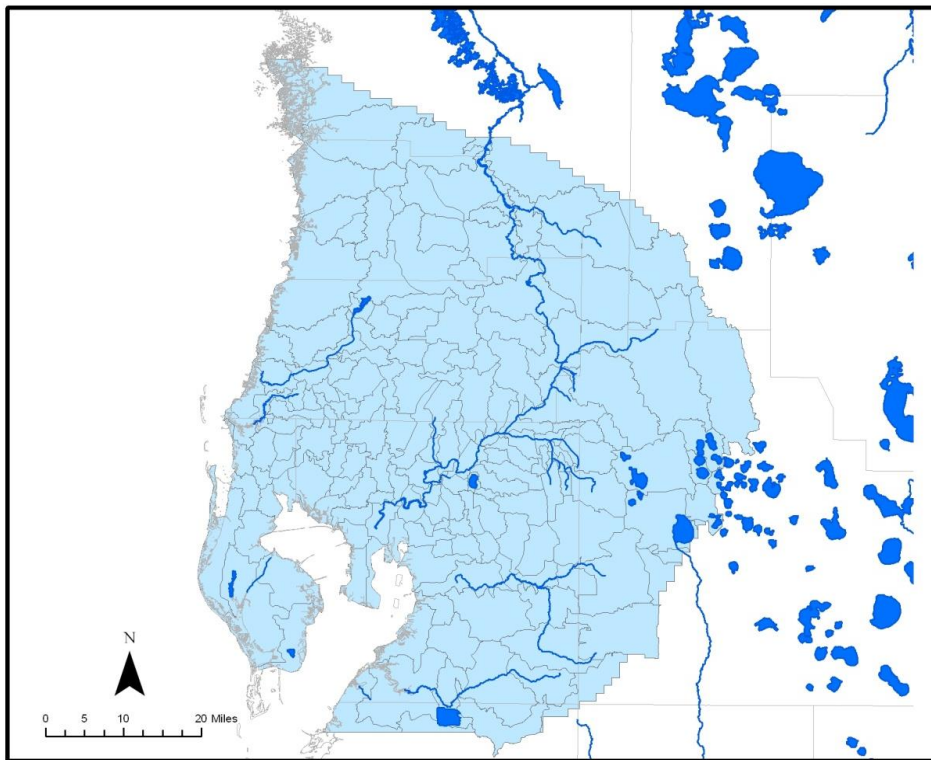


Figure 3. HSPF subbasins in the INTB model.

The INTB model is a regional simulation and has been calibrated to meet global metrics. The model is calibrated using a daily integration step for a transient 10-year period from 1989-1998. A model Verification period from 1999 through 2006 was also added. Model-wide mean error for all wells in both the surficial and Upper Floridan aquifers is less than 0.2 feet during both the calibration and verification periods. Mean absolute error was less than two feet for

both the surficial and Upper Floridan aquifer. Total stream flow and spring flow mean error averaged for the model domain is each less than 10 percent. More information summarizing the INTB model calibration can be found in Geurink and Basso (2012).

INTB Model Scenarios

Three different groundwater withdrawal scenarios were run with the INTB model. The first scenario consisted of simulating all groundwater withdrawn within the model domain from 1989 through 2000. The second scenario consisted of eliminating all pumping in the Central West-Central Florida Groundwater Basin (Figure 4). Total withdrawals within the Central West-Central Florida Groundwater Basin averaged 239.4 mgd during the 1989-2000 period. TBW central wellfield system withdrawals were simulated at their actual withdrawal rates during this period. The third scenario consisted of reducing TBW central wellfield system withdrawals to their mandated recovery quantity of 90 mgd from the 11 central system wellfields. For TBW only, the 2008 pumping distribution was adjusted slightly upward from 86.9 mgd to 90 mgd to match recovery quantities.

Taking the difference in simulated heads from the 1989-2000 pumping to non-pumping runs, the average predicted drawdown was 0.5 ft in the both the surficial and Upper Floridan aquifers near Pierce Lake (Figure 5 and 6). Taking the difference in modeled heads from the TBW recovery pumping to non-pumping runs, the average predicted drawdown was 0.2 ft in the both the surficial and Upper Floridan aquifers near Pierce Lake (Figure 6 and 7). Table 1 presents the predicted drawdown in the surficial and the Upper Floridan aquifer based on the INTB model results.

Table 1. INTB model results for Pierce Lake.

Lake Name	Predicted Drawdown (ft) in the Surficial Aquifer due to 1989-2000 Withdrawals*	Predicted Drawdown (ft) in the Surficial Aquifer with TBW Withdrawals reduced to 90 mgd*
Pierce	0.5	0.2
Lake Name	Predicted Drawdown (ft) in the Upper Floridan Aquifer due to 1989-2000 Withdrawals*	Predicted Drawdown (ft) in the Upper Floridan Aquifer with TBW Withdrawals reduced to 90 mgd*
Pierce	0.5	0.2

* Average drawdown from model cells intersecting lake

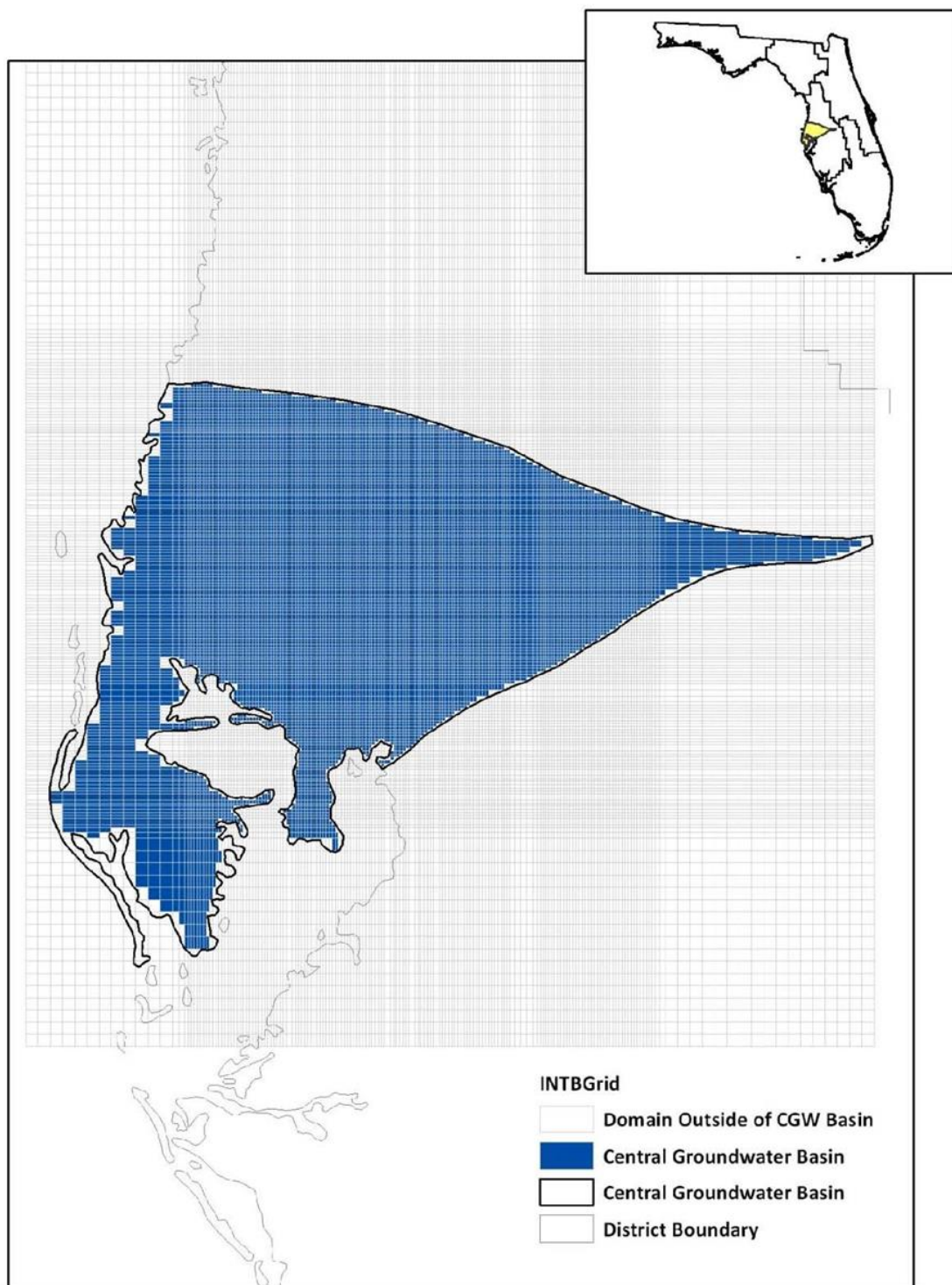


Figure 4. INTB scenarios where impacts to the hydrologic system were simulated due to groundwater withdrawals in the Central West-Central Florida Groundwater Basin.

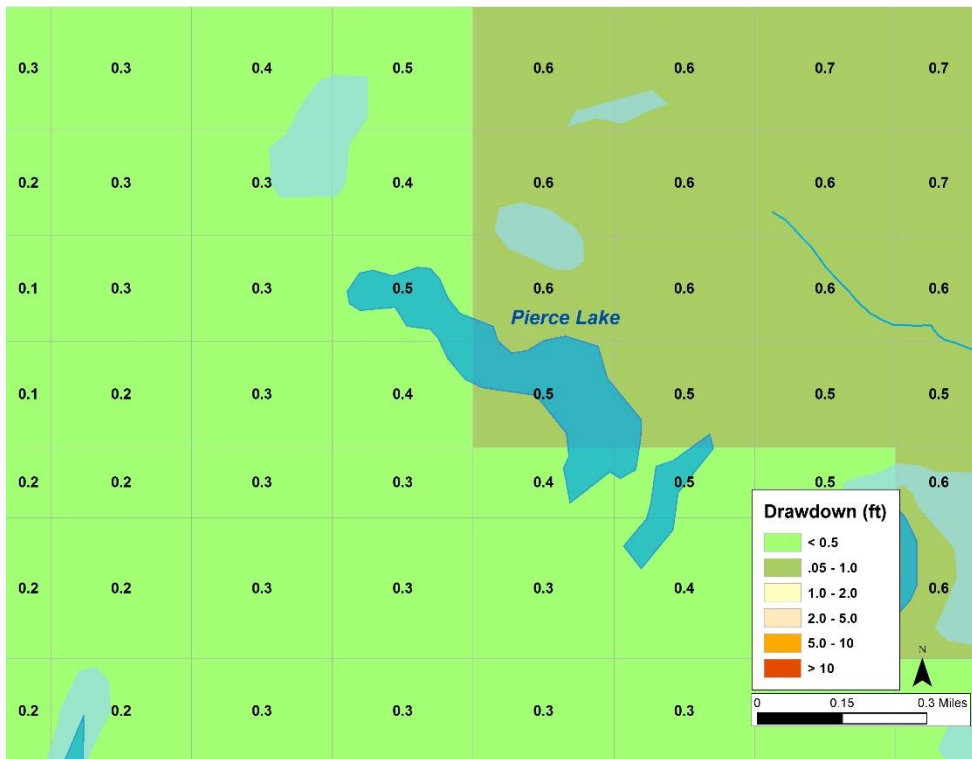


Figure 5. Predicted mean drawdown in the surficial aquifer due to 1989-2000 groundwater withdrawals.

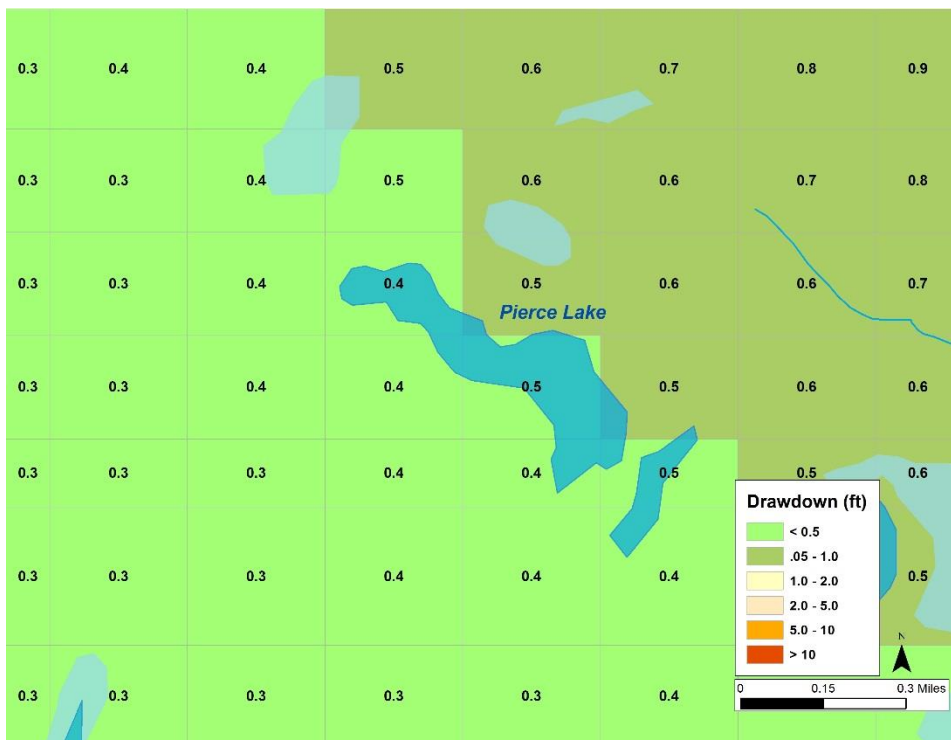


Figure 6. Predicted mean drawdown in the Upper Floridan aquifer due to 1989-2000 groundwater withdrawals.

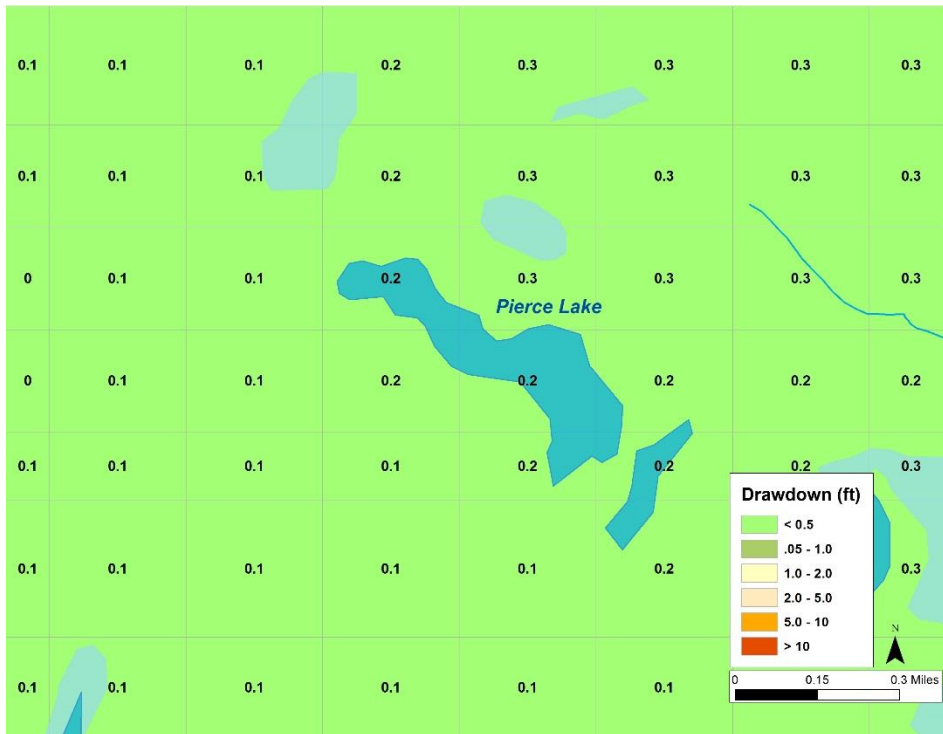


Figure 7. Predicted mean drawdown in the surficial aquifer due to TBW 90 mgd groundwater withdrawals.

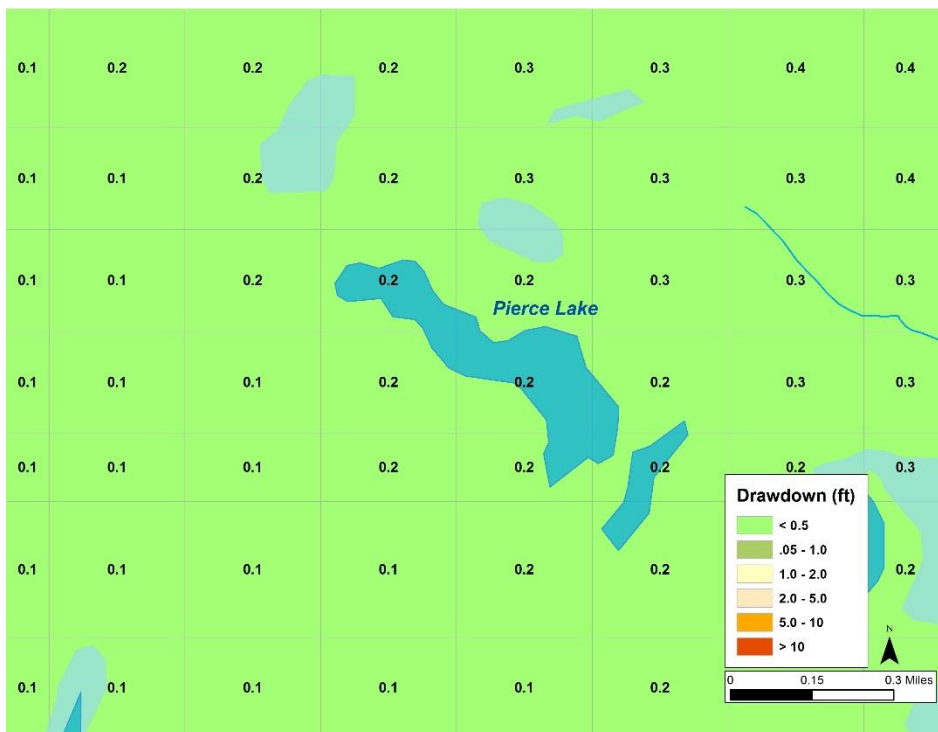


Figure 8. Predicted mean drawdown in the Upper Floridan aquifer due to TBW 90 mgd groundwater withdrawals.

D. References

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