

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

ENVIRONMENTAL RESOURCE PERMIT APPLICANT'S HANDBOOK VOLUME II

DESIGN REQUIREMENTS FOR STORMWATER TREATMENT AND MANAGEMENT SYSTEMS WATER QUALITY AND WATER QUANTITY

**FOR USE WITHIN THE GEOGRAPHIC LIMITS OF THE SOUTHWEST FLORIDA
WATER MANAGEMENT DISTRICT**

EFFECTIVE [SPRING 2013]

**[to be effective same date as Chapter 62-330, F.A.C. Statewide ERP
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Volume II is incorporated by reference in 40D-4.091, F.A.C.

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PART I – INTRODUCTION, ORGANIZATION, APPLICABILITY

1.0 Introduction

To assist applicants seeking Environmental Resource Permits (ERPs), an Applicant's Handbook has been prepared as part of the overall effort to promote greater statewide consistency in the administration of Chapter 62-330, Florida Administrative Code (F.A.C.). The ERP Applicant's Handbook is presented in two volumes. **Applicant's Handbook Volume I** (General and Environmental), is applicable statewide and contains the following:

- Background information on the ERP program, including points of contact;
- A summary of the statutes and rules that are used to authorize and implement the ERP program;
- A summary of the types of permits, permit thresholds, and exemptions;
- A discussion of the environmental criteria used for ERP evaluations;
- A discussion of the erosion and sediment control requirements for ERP projects, and
- A discussion of requirements for system operation and maintenance.

Each Water Management District has adopted an **ERP Applicant's Handbook Volume II** which contains the District-specific design and performance criteria for stormwater quantity, flood control, stormwater quality and any special basin criteria or other requirements that are applicable within the geographic area of the specific water management district. This **ERP Applicant's Handbook Volume II** (Design Requirements for Stormwater Treatment and Management Systems – Water Quality and Water Quantity) is intended for use only within the jurisdictional boundaries of the Southwest Florida Water Management District.

Together, **Applicant's Handbook Volumes I and II (AH I and II)** set forth the usual procedures and information used by District staff in the review of permit applications. The overall objective of the review is to ensure that the activities authorized by an ERP are not harmful to the water resources of the District and not inconsistent with the public interest or the overall objectives of the District.

This **AH II** is intended to be applicable to those types of projects that involve stormwater management systems that consist of more than just incidental dredging or filling and which require an individual permit or authorization pursuant to Section 403.814(12), F.S. ("10-2 Permits."). Many minor "stand-alone" activities or works generally will not give rise to water quantity, flood control or water quality concerns that must be addressed in accordance with the performance standards and design criteria set forth in this **AH II**. However, if a project requires consideration of water quantity, water quality or flood impacts and specific measures or design

features in order to demonstrate reasonable assurance that all required conditions for permit issuance have been met, this **AH II** will be applicable.

This **AH II** provides specific, detailed information to help applicants meet the water quality, water quantity, flood control, construction and design requirements applicable within this District. It is incorporated by reference in Rule 40D-4.091, F.A.C., as well as in Chapter 62-330, F.A.C., and, as such, **AH II** constitutes rules of the DEP and the District. The term “Agency” or “District,” when used in the **AH I or II** or in Chapter 62-330, F.A.C., shall refer to the DEP, this District, all Water Management Districts or a delegated local government, as applicable, in accordance with the division of responsibilities specified in the Operating Agreements incorporated by reference in subsection 62-330.010(5), F.A.C., except where a specific agency is otherwise identified. **AH I and II** are written to provide more detail and clarity for the public in understanding the statutory and rule provisions that implement the ERP program, and are intended to be written in an understandable, “user-friendly” format.

Pursuant to Subsection 373.4131(1)(c), F.S., the statewide ERP rules set forth in Chapter 62-330, F.A.C., are to rely primarily on the rules of the DEP and water management districts in effect immediately prior to the effective date of the new statewide rules. Accordingly, where applicable, a history note is provided for the various sections and paragraphs of this **AH II** which identifies the source of the particular provision as being the District’s Environmental Resource Permitting Information Manual Part B, Basis of Review (BOR) (effective date 12/29/2011) or in some cases the Northwest Florida Water Management District (NFWFMD) Applicant’s Handbook Volume I or Volume II (effective date 11/20/2010) or other source as applicable. Most of the provisions of this **AH II** contain material transferred directly from Chapters 1, 3 through 6 of the District’s ERP BOR, with no substantive changes or only minimal changes necessary to address standardized formatting or to reference related provisions in Chapter 62-330, F.A.C. or in **AH I**. To promote a more consistent statewide approach, the NFWFMD Applicant’s Handbook, drafted and adopted by DEP for use within that District, served as the model for the development of the statewide **AH I and II**. Where appropriate, provisions contained in NFWFMD’s Applicant’s Handbook that described the same practice or approach used in this District for addressing water quality, water quantity or flood control requirements were included in this **AH II**. Additional provisions were also added pursuant to guidance from DEP, to promote statewide consistency.

History Note: Adapted from NFWFMD Applicant’s Handbook Volume II, Part I.

1.1 Objectives.

Pursuant to Part IV of Chapter 373, F.S., and Chapter 62-330, F.A.C., the District is responsible for permitting the construction, alteration, operation, maintenance, repair, abandonment or removal of surface water management systems within its jurisdictional boundaries, in accordance with its Operating Agreement with DEP incorporated by reference in Section 62-330.010(5), F.A.C. The objective of this Applicant’s Handbook, Volumes I and II is to identify the usual procedures and information used by the District in permit application review. The objective of the review is to ensure that the permit will authorize activities or situations which are

not harmful to the water resources of the District nor inconsistent with the public interest or the overall objectives of the District.

History Note: Transferred from SWFWMD Environmental Resource Permitting Information Manual, Part B, Basis of Review, section 1.1.

1.2 Thresholds.

Thresholds for permitting are set forth in subsection 62-330.020(2), F.A.C., and apply statewide. There are currently no additional District-specific thresholds applicable within this District. If any are established in the future, they will be set forth in this section.

History Note: New

1.3 District-Specific Exemptions.

In addition to the exemptions set forth in Section 62-330.051, F.A.C., the specific activities described below are exempt from the requirement to obtain an ERP in this District:

- (1) The operation and maintenance of a surface water management system which:
 - (a) Was constructed before October 1, 1984; or
 - (b) Was constructed or was being constructed on or before December 9, 1999, and was not required to obtain a District permit under exemptions existing at the time.
- (2) The following mining activities:
 - (a) Any system for a mining or mining related activity which has a valid permit issued by the District or the Department pursuant to Rule 40D-45.041, F.A.C. This exemption shall be for the plans, terms and conditions approved in the permit issued pursuant to Chapter 40D-45, F.A.C. If an operator of a system previously permitted under Chapter 40D-45, F.A.C., proposes to alter such system, the alteration shall be reviewed under the provisions of this chapter.
 - (b) Phosphate mining, phosphate mining related surface water management systems, and reclamation and restoration conducted in accordance with Chapter 62C-16, F.A.C., within the District, provided that all the following conditions are met.
 1. Activities associated with mining operations as defined by and subject to Sections 378.201 through .212, F.S., and included in a conceptual reclamation plan or modification application submitted prior to July 1, 1996, shall continue to be exempt under this subsection.
 2. The location of any existing point of discharge authorized in a previous permit issued by the Department, the Department of Environmental Regulation, or the District shall not be changed, and the volume and frequency of such discharge shall not be exceeded.
 3. Natural drainage from off-site up gradient areas shall not be interrupted so as to cause damage to off-site property or the public, and natural drainage patterns on undisturbed lands shall be maintained to the maximum extent achievable without adversely altering the time, stage, volume and point or manner of discharge or dispersion.
- (3) Proposed normal and necessary farming operations as are customary for the area that can be conducted in an environmentally sustainable manner, provided such operations and facilities:

- (a) Do not cause adverse water quantity or offsite flooding impacts;
- (b) Do not involve activities in wetlands or other surface waters for which mitigation would be required; and
- (c) Do not adversely impact water quality in offsite receiving waters.

Persons desiring to qualify for this exemption should submit site drainage and conservation plans for the proposed normal and necessary farming operations which incorporate Natural Resource Conservation Service, Florida Department of Agriculture and Consumer Services, or equivalent conservation standards or best management practices in accordance with Section 1.3.2 below. Following a meeting with District agricultural regulatory staff and verification that the operations, facilities, and plans comply with paragraphs (a) through (c), above, the District will provide written notice of the exemption, if qualified.

History note: (1) transferred from 40D-4.051(2); (2) transferred from 40D-4.051(5) and 40D-4.053; and (3) transferred from 40D-4.051(4) with amendments; F.A.C.

1.3.1 Agricultural Exemption Determinations Available Through the District's Agricultural Surface and Ground Water Management System Program.

Historically, the construction, alteration, operation, maintenance (excluding routine custodial maintenance), abandonment or removal of agricultural surface water management systems has required an Environmental Resource Permit (ERP) unless expressly exempt by statute or rule. Many agricultural operations are exempt pursuant to the statutory exemption set forth in subsection 373.406(2), F.S. Additionally, since 1990, the District has implemented a rule-specific agricultural exemption formerly expressed in subsection 40D-4.051(4), F.A.C., an updated version of which is now set forth in AH II Section 1.3(3) above. This exemption provision has been updated to align with amendments to the statutory agricultural exemption that became effective July 1, 2011. For many years the District has also provided services and resources to assist farmers and other agriculturalists in meeting environmental and agricultural design requirements through incentive-based and ecosystem-based resource management practices. These services and exemption determinations continue to be provided through the District's Agricultural Surface and Ground Water Management System (AGSWM) program, which promotes voluntary implementation of best management practices (BMPs) and other environmentally beneficial farming principles as a passive alternative to environmental resource permitting.

The District's AGSWM program relies upon technical assistance available from the United States Department of Agriculture Natural Resources Conservation Service (NRCS) that encourages agriculturalists to use resource management system (RMS) conservation planning and to practice good water management. The NRCS specializes in RMS conservation planning, which may provide farmers with a viable alternative to the usual permitting procedures. The District's Ag Team, which consists of professional engineering and environmental staff who specialize in agricultural operations, is available to offer assistance to farmers seeking either verification of exemption from ERP requirements or other on-site review and guidance regarding sustainable agricultural practices. Conservation planning techniques of the NRCS further complement District Ag Team efforts to help facilitate surface water and water use regulation (permitting or exemption) for qualifying agricultural projects.

1.3.2 Process for Obtaining Agricultural Exemptions.

The District will continue to provide confirmation of qualification of exemption from permitting through the District's voluntary AGSWM program for farmers desiring to avail themselves of the District's specific agricultural-related exemption, as well as confirmation of exemption pursuant to the statutory exemption set forth in section 373.406(2), F.S., and any other applicable statutory or rule exemption for agricultural activities. Written requests for verification of exemption must comply with the requirements of section 62-330.050, F.A.C., and must include the fee specified in section 40D-1.607, F.A.C.

Farmers seeking an agricultural exemption determination are encouraged to contact the District's Ag Team as a first step. The District's Ag Team is based in the Tampa Permitting Office and is available for meetings in any of the District's service offices. Persons desiring to qualify for the exemption set forth in AH II section 1.3(3) will be expected to submit appropriate site-specific drainage and conservation plans for the proposed operations and demonstrate adherence to applicable nutrient, pest, drainage, irrigation or other conservation standards and BMPs that are adopted or recognized by NRCS, the Florida Department of Agriculture and Consumer Services (FDACS), or other equivalent source. The grower may contact the NRCS to obtain a federally prescribed RMS plan of site specific BMPs that may be used as part of the District's agricultural exemption confirmation process. The local NRCS office for specific regions may be found at <http://www.fl.nrcs.usda.gov/contact/index.html>. FDACS' Office of Agricultural Water Policy also has adopted by rule certain statewide BMP manuals for major commodity crops such as citrus, container nurseries, cow/calf operations, sod, vegetable and agronomic crops, and specialty fruit and nut crops. Implementation of the FDACS-prescribed BMPs provides a presumption of compliance with statewide water quality discharge standards. A listing of FDACS-adopted BMPs and links to the FDACS website for reviewing the available BMPs and manuals is contained in Appendix E attached to this AH II.

Following an on-site meeting with District agricultural regulatory staff, review of submitted material and confirmation that the proposed operations, facilities, and plans will comply with the provisions of AH II section 1.3(3) above, the District will provide written notice of verification of exemption.

Exemption from permitting for agricultural activities is also established pursuant to subsection 373.406(2), F.S., (known as the statutory agricultural exemption). This provision allows persons engaged in the occupation of agriculture, silviculture, floriculture or horticulture to alter the topography of any tract of land, including but not limited to activities that may impede or divert the flow of surface waters or adversely impact wetlands, for purposes consistent with the normal and customary practice of such occupation in the area; provided that such alteration is not for the sole or predominant purpose of impeding or diverting the flow of surface waters or adversely impacting wetlands. This exemption applies to lands classified as agricultural pursuant to section 193.461, F.S., and to activities requiring an ERP pursuant to Part IV of Chapter 373, F.S. This exemption does not apply to any activities previously authorized by an ERP or a management and storage of surface waters permit pursuant to Part IV of Chapter 373, F.S., or a dredge and fill permit issued pursuant to Chapter 403, F.S. While a District determination of exemption from permitting on the basis of this statutory exemption is not required in order for

such activities to be exempt, the AGSWM program can be used to obtain District verification of this exemption.

1.4 Criteria and Flexibility.

The criteria contained in this AH II were established with the primary goal of meeting District water resource objectives as set forth in Chapter 373, F.S. Performance criteria are used where possible. However, the criteria set forth in this AH II are designed to be flexible. Other methods of meeting the overall objectives and the conditions for issuance set forth in Rules 62-330.301 and 62-330.302, F.A.C., will be considered depending on the magnitude of specific or cumulative impacts. Reasonable assurance in the form of plans, test results, or other information must be provided by the applicant to demonstrate that the alternative design meets the conditions for permit issuance.

Compliance with the criteria herein constitutes a presumption that the proposed activity is in conformance with the conditions for issuance set forth in Rules 62-330.301 and 62-330.302, F.A.C. Pursuant to Section 373.4131, F.S., if a stormwater management system is designed in accordance with the criteria in this Handbook or if a system is constructed, operated, and maintained for stormwater treatment in accordance with a valid Environmental Resource Permit or exemption under Part IV of Chapter 373, the discharges from the system are presumed not to violate applicable state water quality standards.

History Note: Transferred from SWFWMD Environmental Resource Permitting Information Manual, Part B, Basis of Review, section 1.3.

1.5 Simultaneous Reviews.

Aside from purely technical aspects, legal and institutional factors also must be considered. Because of legal time constraints for processing permits, it is advisable for the applicant to contact other interested agencies, organizations, and affected citizens prior to submitting a formal application to the District. Summaries of meetings and copies of responses from appropriate parties should be included in the application.

It may be in the applicant's best interest to seek simultaneous reviews from all agencies with jurisdiction. This provision is not intended to preclude the submission of an application to this District prior to receiving other necessary approvals. However, coordinating the review of this application with all appropriate agencies of local government will help ensure that the final design approved by the District meets the requirements of all agencies. Applicants should note the possibility that additional requirements from agencies of local government not contained within the final approved design may necessitate a permit modification.

Issuance of an Environmental Resource Permit by the District does not relieve the applicant of the responsibility to obtain all necessary federal, state, local or special district permits or authorizations.

1/22/2013 Draft

History Note: Transferred from SWFWMD ERP Information Manual Part B, Basis of Review, Section 1.4, with amendments.

PART II — GENERAL CRITERIA

2.0 General Design and Performance Criteria for all Stormwater Management Systems.

This AH Volume II applies to the design of stormwater management systems that require a permit under Chapter 62-330, F.A.C., other than systems that qualify for a general permit, and applies to projects that qualify for a "10/2" permit. All stormwater management systems must be designed, constructed, operated and maintained in accordance with the stormwater quality criteria and stormwater quantity/flood control criteria set forth in this AH II.

History Note: Adapted from NFWFMD AH II, sections 2.0 and 2.1.

2.1 Definitions and Terms.

The following terms are addressed in this AH II and apply within the Southwest Florida Water Management District. These terms are in addition to the definitions and terms that apply statewide and which are provided in AH I or in Chapter 62-330, F.A.C. or applicable statutes:

2.1.1 "Aquitard"

A tightly compacted soil structure that retards but does not prevent flow of water to or from an adjacent aquifer. It does not allow water to pass through it fast enough to be used as a water supply, but if breached, could allow mixing of water sources between adjacent aquifers.

2.1.2 "Closed Drainage Basin"

A drainage basin in which the runoff does not have a surface outfall up to and including the 100-year flood level.

2.1.3 "Control Device"

The element of a discharge structure which allows the gradual release of water under controlled conditions. This is sometimes referred to as the bleed-down mechanism or "bleeder." Examples include orifices, notches, weirs, and effluent filtration systems.

2.1.4 "Control Elevation"

The lowest elevation at which water can be released through the control device. This is sometimes referred to as the invert elevation.

2.1.5 "Detention"

The delay of storm runoff prior to discharge into receiving waters.

2.1.6 "Detention Volume"

The volume of open surface storage behind the discharge structure measured between the overflow elevation and control elevation.

2.1.7 "Directly Connected Impervious Areas"

Unless otherwise specifically stated in this AH II, directly connected impervious areas as considered in the calculation of volumes for treatment systems are those impervious and semi-impervious areas hydraulically connected to the treatment system directly or by pipes or ditches.

2.1.8 "Discharge Structure"

A structural device, usually of concrete, metal, etc., through which water is discharged from a project to the receiving water.

2.1.9 "Drainage Basin"

A subdivision of a watershed. District drainage basins are provided in section 2.7.

2.1.10 "Elevation"

The height in feet above mean sea level according to the appropriate established vertical data, such as North American Vertical Datum (NAVD) or National Geodetic Vertical Datum (NGVD).

2.1.11 "Historic Basin Storage"

The depression storage available on the site in the pre-development condition. The volume of storage is that which exists up to the required design storm.

2.1.12 "Historic Discharge"

The peak rate and/or amount of runoff which leaves a parcel of land by gravity from an undisturbed/existing site, or the legally allowable discharge at the time of permit application.

2.1.13 "Hydroperiod"

The duration of inundation in a wetland.

2.1.14 "Normal Water Level"

The design starting water elevation used when determining stage/storage design computations in a retention or detention area. A retention or detention system may have two (2) designated "normal water levels" associated with it if the system is designed for both water quality and water quantity.

2.1.15 "Off-line Treatment System"

A system only for water quality treatment that collects project runoff and has no direct discharge capability other than percolation and evaporation. Off-line treatment systems provide storage of the treatment volume off-line from the primary conveyance path of flood discharges. A system utilizing detention with effluent filtration is not an off-line treatment system.

2.1.16 "On-line Treatment System"

A dual purpose system that collects project runoff for both water quality and water quantity requirements. Water quality volumes can be recovered through percolation, evaporation, filtration or detention.

2.1.17 "Open Drainage Basin"

Open drainage basins are all basins not meeting the definition of a closed drainage basin.

2.1.18 "Overflow Elevation"

The design elevation of a discharge structure at or below which water is contained behind the structure, except for that which leaks or bleeds out, through a control device down to the control elevation.

2.1.19 "Regulated Activity"

The construction, alteration, operation, maintenance, abandonment or removal of a system regulated pursuant to Part IV, Chapter 373, F.S., or Part V, Chapter 403, F.S.

2.1.20 "Surface Water or Stormwater Management System Facilities"

All components of a permitted surface water or stormwater management system including but not limited to all inlets, ditches, culverts, water control structures, retention and detention areas, ponds, lakes, floodplain compensation areas, wetlands and other surface waters and any associated buffer areas, and wetland mitigation areas.

2.1.21 "Water Management Areas"

Areas to be utilized for the conveyance or storage of surface water, mitigation, or perpetual operation and maintenance purposes.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, section 1.7 (duplicative definitions deleted); 2.1.21 transferred from Basis of Review section 2.6.2.2.5.

2.2 Criteria for Evaluation – Reasonable Assurance.

All permit applicants must provide reasonable assurance that a surface water management system will meet the criteria in Rule 62-330.301, F.A.C. For activities proposed to be located in, on or over wetlands or other surface waters, the additional criteria set forth in Rule 62-330.302, F.A.C., must also be met. Applications involving mitigation banks must also meet the criteria established in Chapter 62-342, F.A.C. The conditions for issuance for a permit set forth in Rule 62-330.301, F.A.C., are as follows:

- (a) Will not cause adverse water quantity impacts to receiving waters and adjacent lands;
- (b) Will not cause adverse flooding to on-site or off-site property;
- (c) Will not cause adverse impacts to existing surface water storage and conveyance capabilities;
- (d) Will not adversely impact the value of functions provided to fish and wildlife and listed species by wetlands and other surface waters;
- (e) Will not adversely affect the quality of receiving waters such that the water quality standards set forth in Chapters 62-4, 62-302, 62-520, and 62-550, F.A.C., including the antidegradation provisions of paragraphs 62-4.242(1)(a) and (b), F.A.C., subsections 62-4.242(2) and (3), F.A.C., and Rule 62-302.300, F.A.C., and any special standards for Outstanding Florida Waters (OFWs) and Outstanding National Resource Waters (ONRWs) set forth in subsections 62-4.242(2) and (3), F.A.C., will be violated;
- (f) Will not cause adverse secondary impacts to the water resources. In addition to the criteria in this subsection and in subsection 62-330.301(2), F.A.C., in accordance with Section 373.4132, F.S., an applicant proposing the construction, alteration, operation, maintenance, abandonment, or removal of a dry storage facility for 10 or more vessels that is functionally associated with a boat launching area must also provide reasonable assurance that the facility, taking into consideration any secondary impacts, will meet the provisions of paragraph 62-330.302(1)(a), F.A.C., including the potential adverse impacts to manatees;
- (g) Will not adversely impact the maintenance of surface or ground water levels or surface water flows established pursuant to Section 373.042, F.S., as set forth in Chapter 40D-8, F.A.C.;
- (h) Will not cause adverse impacts to a work of the District established pursuant to Section 373.086, F.S.;
- (i) Will be capable, based on generally accepted engineering and scientific principles, of being performed and of functioning as proposed;

- (j) Will be conducted by an entity with the financial, legal, and administrative capability of ensuring that the activity will be undertaken in accordance with the terms and conditions of the permit, if issued; and
- (h) Will comply with any applicable special basin or geographic area criteria rules incorporated by reference in Chapter 62-330, F.A.C.

The standards and criteria contained in the Applicant's Handbook Volumes I and II shall determine whether the reasonable assurances required by subsections 62-330.301 and .302, F.A.C., have been provided.

History Note: Section 62-330.301, F.A.C. (Effective date); also transferred from 40D-4.301(1) and (3), F.A.C.

2.3 Professional Certification.

All construction plans, reports, specifications and supporting calculations submitted to the District for stormwater management systems that require the services of a registered professional must be signed, sealed, and dated by such registered professional. A "registered professional" is defined in Applicant's Handbook Volume I section 2.0(a)82.

History Note: Adapted from NFWMD AH II section 2.3.

2.4 Tailwater Considerations.

"Tailwater" refers to the receiving water elevation (or pressure) at the final discharge point of the stormwater management system. Tailwater is an important component of the design and operation of nearly all stormwater management systems and can affect any of the following management objectives of the system:

- (a) Peak discharge from the stormwater management system;
- (b) Peak stage in the stormwater management system;
- (c) Level of flood protection in the project;
- (d) Recovery of peak attenuation and stormwater treatment volumes; and
- (e) Control elevations, normal water elevation regulation schedules, and ground water management.

History Note: Adapted from NFWMD AH II section 2.7.

2.4.1 Tailwater For Water Quality Design.

Stormwater management systems designed in accordance with the water quality design provisions in Part III of this AH II must provide a gravity or pumped discharge that effectively operates (i.e., meets applicable rule criteria) under tailwater conditions. Acceptable criteria for demonstrating effective tailwater conditions include such criteria as mean annual high tide for tidal areas and mean annual wet-season high water elevation.

History Note: Adapted from NFWMD AH II section 2.7.1.

2.4.2 Tailwater for Water Quantity Design.

Stormwater management systems designed in accordance with the water quantity provisions of Part IV of this AH II must consider tailwater conditions. Receiving water stage can affect the amount of flow that will discharge from the project to the receiving water. Applicants are advised to use an appropriate time-stage relationship for a storm equal to the project design storm. Variable tailwater stages should be considered if they have a significant influence on the design.

History Note: adapted from NFWMD AH II section 2.7.1; last sentence transferred from BOR 7.7.3

2.4.3 Regulated Systems.

Design and maintained stage elevations should be available either from the local jurisdiction or the District. Stages for frequencies other than the design will be estimated by the District upon request from the applicant.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.7.1.

2.5 Retrofits of Existing Stormwater Water Management Systems.

A retrofit project can be undertaken, generally in urbanized areas, for purposes of improving or addressing water quality treatment, water quantity or flooding concerns or for combined purposes. A water quality retrofit project typically adds treatment to an existing stormwater management system or systems and results in reduced stormwater pollutant loadings from the existing system or systems. A water quantity retrofit project typically adds attenuation, storage volume or otherwise addresses flooding concerns. Retrofit projects are not intended to serve new development or redevelopment. The applicant for a water quality or water quantity retrofit project must provide reasonable assurance that the proposed project activities will not result in any new or additional adverse water quality or quantity impacts to receiving waters.

For water quality retrofit projects, if the applicant has conducted, and the District has approved, an analysis that provides reasonable assurance that the proposed water quality retrofit will provide the intended pollutant load reduction from the existing system or systems, the project will be presumed to comply with the conditions for issuance.

Water quantity retrofit projects proposed to reduce existing flooding problems must be designed in such a way that the project does not reduce stormwater treatment capacity or increase discharges of untreated stormwater. Where existing ambient water quality does not meet water quality standards and the proposed activities will contribute to this existing condition, measures must be proposed that will provide a net improvement of the water quality for those parameters that do not meet standards.

Applicants are advised that a conceptual approval permit and general permit are also available pursuant to sections 62-330.055 and 62-330.450, F.A.C., for certain urban infill and redevelopment projects undertaken by local governments.

History Note: Derived from NFWMD Applicant's Handbook Volume II, section 2.10

2.6 District Drainage Basins and Watersheds.

Pursuant to paragraph 62-330.302(1)(b), F.A.C., cumulative impacts upon wetlands and other surface waters are analyzed by evaluating impacts to water quality and functions provided by wetlands and other surface waters within the same drainage basin. A regulated activity shall not cause unacceptable cumulative impacts upon wetlands and other surface waters within the same drainage basin as the regulated activity for which a permit is sought. Further information on cumulative impact assessment appears in sections 10.1 and 10.2 of AH I. The District's adopted drainage basins for cumulative impact analysis and watersheds for mitigation bank purposes are the same and are set forth in Figure 2.6 which follows at the end of this chapter.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review Appendix 6 and Appendix 4 Exhibit 1

2.7 Flexibility for State Transportation Projects and Facilities.

Due to the unique limitations of state linear transportation projects and facilities, subsection 373.413(6), F.S. (2012), requires the Agency, during the review of such activities, to consider and balance the expenditure of public funds for stormwater treatment with the benefits to the public in providing the most cost-efficient and effective method of achieving the treatment objectives of stormwater management systems. To do so, alternatives to onsite treatment for water quality will be considered, which may include regional stormwater treatment systems.

1/22/2013 Draft

Figure 2.6 – *on separate page to be numbered 15.*



Basin_Watershed_Map_SWFWMD.pdf

PART III -- STORMWATER QUANTITY/FLOOD CONTROL

3.0 General Stormwater Quantity and Flood Control Requirements.

Pursuant to the Conditions for Issuance in Section 62-330.301, F.A.C., an applicant must provide reasonable assurance that the proposed construction, alteration, operation, maintenance, removal or abandonment of the works or other activities regulated under ERP rules:

- a. Will not cause adverse water quantity impacts to receiving waters and adjacent lands;
- b. Will not cause adverse flooding to on-site or off-site property;
- c. Will not cause adverse impacts to existing surface water storage and conveyance capabilities; and
- d. Will not adversely impact the maintenance of surface or ground water levels or surface water flows established pursuant to Section 373.042, F.S., or Chapter 40D-8, F.A.C.

Utilization of the design criteria in this Part III will provide reasonable assurance of compliance with these conditions for issuance unless credible historical evidence of past flooding or the physical capacity of the downstream conveyance or receiving waters indicates that the conditions for issuance will not be met without consideration of storm events of different frequency or duration. In those instances, applicants shall be required to provide additional analyses using storm events of different duration or frequency than those referenced below, or to adjust the volume, rate or timing of discharges, to provide reasonable assurance of compliance with the conditions for issuance. Pre-application meetings are encouraged for projects in flood-prone areas to determine whether additional analysis is necessary to demonstrate reasonable assurance of compliance with the conditions for issuance.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.1

3.1 Discharges.

Off-site discharge is limited to amounts which will not cause adverse off-site impacts.

a. For a project or portion of a project located within an open drainage basin, the allowable discharge is:

1. historic discharge, which is the peak rate at which runoff leaves a parcel of land by gravity under existing site conditions, or the legally allowable discharge at the time of permit application; or

2. amounts determined in previous District permit actions.

b. Except in situations as described in Section 3.0 above, off-site discharges and peak stages for the existing and developed conditions shall be computed using the Southwest Florida Water Management District's 24-hour, 25-year rainfall maps and the Natural Resources

Conservation Service type II Florida Modified 24-hour rainfall distribution with an antecedent moisture condition II. See Appendix A for these items.

c. For a project or portion of a project discharging to a tidal water body, the peak discharge requirements of this section are not required, provided that the rate of discharge does not cause adverse impacts. Examples of tidal water bodies are the Gulf of Mexico and the Gulf Intracoastal Waterway, including manmade portions of the Gulf Intracoastal Waterway.

d. For a project or portion of a project located within a closed drainage basin, the required retention volume shall be the post-development runoff volume less the pre-development runoff volume computed using the Southwest Florida Water Management District's 24-hour/100-year rainfall map and the Natural Resources Conservation Service type II Florida Modified 24-hour rainfall distribution with an antecedent moisture condition II. The total post development volume leaving the site shall be no more than the total pre-development volume leaving the site for the design 100-year storm. The rate of runoff leaving the site shall not cause adverse off-site impacts. Maintenance of pre-development off-site low flow may be required in hydrologically sensitive areas.

e. When not in conflict with the objectives of recharge, dewatering, or maintaining ground water levels, projects serviced by a permitted or approved regional surface water management system may discharge stormwater runoff at the rate and volume established by the agency operating the regional stormwater system. The permittee must provide written verification from the operating agency stating the acceptable rate and volume of stormwater runoff from the project.

f. In no case shall the proposed surface water management system be required to account for storm events less frequent than the 25 year event in an open basin or the 100-year event in a closed basin.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.2. (subsection c is added).

3.2 Flood Protection.

Flood protection for structures should be provided as follows:

Residential buildings should have the lowest floor elevated above the 100-year flood elevation for that site.

Industrial, commercial or other non-residential buildings susceptible to flood damage should have the lowest floor elevated above the 100-year flood elevation or be designed and constructed so that below the 100-year flood elevation the structure and attendant utility facilities are watertight and capable of resisting the effects of the regulatory flood. The design should take into account flood velocities, duration, rate of rise, hydrostatic and hydrodynamic forces, the effect of buoyancy and impacts from debris. Flood proofing measures should be operable without human intervention and without an outside source of electricity.

Accessory buildings may be constructed below the 100-year flood elevation provided there is minimal potential for significant damage by flooding.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.3.

3.3 Flood Plain Encroachment.

No net encroachment into the flood plain, up to that encompassed by the 100-year event, which will adversely affect either conveyance, storage, water quality or adjacent lands will be allowed. Any required compensating storage shall be equivalently provided between the seasonal high water level and the 100-year flood level to allow storage function during all lesser flood events.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.4.

3.4 100-Year Flood Level Determinations.

a. Flood elevations shall be determined using the most accurate information available, which can include:

1. Actual data, including water level, stream flow and rainfall records, or
2. Hydrologic/hydraulic modeling, or
3. Federal Flood Insurance Rate Maps and supporting flood study data.
4. Floodplain analysis studies approved by the District Governing Board.

b. Flood elevations shall be evaluated for accuracy considering the extent to which flood elevations are validated by site-specific data.

c. The 24 hour, 100-year storm shall be used to determine the 100-year flood elevation except in those circumstances where credible historical evidence exists that higher flood stages have occurred, and can be expected to re-occur, following more frequent storm events. In those cases, the 100-year flood elevation shall be determined using a 100-year storm of sufficient duration to exceed the flood stages observed following more frequent events.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.4.1.

3.5 Minimum Drainage.

Commercial and industrial projects to be subdivided for sale are required to install a minimum drainage system as described in (a) and (b) below. Projects permitted in such a manner shall require deed restrictions which notify lot or tract purchasers of the amount of additional on-site storm water management system necessary to provide flood attenuation and any additional retention/detention required for water quality purposes.

a. The required water quality system must have treatment capacity for one inch of runoff if wet detention is used, or one-half inch of runoff if retention, effluent filtration or exfiltration is used, from the total developed site and contributing offsite area.

b. A stormwater collection and conveyance system must be provided to interconnect the retention/detention system with the project outfall, including access points to the system available to each individual lot or tract. The system shall be sized to limit discharge under full build-out design conditions to the allowable discharge.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.5.

3.6 Water Conservation, Low Flow and Base Flow Maintenance.

Where practicable, systems shall be designed to:

- a. maintain water tables, base flows and low flows at the highest practicable level. The depth to which the water table can be lowered will be determined based on the potential adverse impact on recharge, the effect on water resources (quality and quantity), and the necessity for fill and its impact on existing natural upland vegetation; and
- b. preserve site environmental values; and
- c. not waste freshwater through overdrainage; and
- d. not lower water tables which would adversely affect existing legal uses; and
- e. preserve site groundwater recharge characteristics; and
- f. retain water on-site for use and re-use for irrigation and other reasonable beneficial uses.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, section 4.6.

3.6.1 Minimum Flows and Levels.

In addition to the design considerations in Section 3.6 above, the system shall not reduce or suppress the flow of a watercourse or the level of water in a wetland or other surface water or the level of ground water below a minimum flow or level that has been established pursuant to Section 373.042, F.S.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.6.1.

3.6.2 Water Withdrawals.

The effects of water withdrawals shall not be considered as the ambient condition in the design of stormwater management systems permitted under Chapter 62-330, F.A.C., except to the extent that the long term success of mitigation would be adversely affected by such water withdrawals.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review Section 4.6.2.

3.7 Historic Basin Storage.

Provision must be made to replace or otherwise mitigate the loss of historic basin storage provided by the project site.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.7.

3.8 Offsite Lands.

The application shall include provisions to allow drainage from off-site upgradient areas to downgradient areas without adversely altering the time, stage, volume, point or manner of discharge or dispersion and without degrading water quality.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.8.

3.9 Isolated Wetlands.

Isolated wetlands wholly owned or controlled by the applicant may be used for flood attenuation purposes when not in conflict with environmental or public use considerations.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 4.9.

3.10 Rural or Minor Subdivisions.

Rural or minor residential subdivisions constructed and operated in accordance with the design and construction criteria specified in AH II Section 5.10 will be presumed to provide reasonable assurance of compliance with the water quantity-related conditions for issuance described in Section 3.0.

History Note: Adapted from 40D-40.301, F.A.C. (9/5/2010).

PART IV -- STORMWATER QUALITY

4.0 Purpose.

Projects shall be designed so that discharges will meet applicable state water quality standards. Projects designed using the criteria found in this section shall be presumed to provide reasonable assurance of compliance with the state water quality standards referenced in Section 62-330.301(1)(e), F.A.C. The applicant may also provide reasonable assurance of compliance with state water quality standards by the use of alternative methods that will provide treatment equivalent to systems designed using the criteria specified in this section. If the applicant chooses to use alternative methods the District will determine whether the applicant has provided reasonable assurance based on information specific to the proposed design and submitted by the applicant.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 5.1.

4.1 Retention and Detention Criteria.

The volume of runoff to be treated from a site shall be determined by the type of treatment system, i.e., wet detention, detention with effluent filtration, on-line treatment system, or off-line treatment system. If off-site run-off is not prevented from combining with on-site runoff prior to treatment, then treatment must be provided for the combined off-site/project runoff.

a. Wet Detention Systems.

1. A wet detention treatment system shall treat one inch of runoff from the contributing area.

2. A manmade wet detention system shall include a minimum of 35 percent littoral zone, concentrated at the outfall, for biological assimilation of pollutants. The percentage of littoral zone is based on the ratio of vegetated littoral zone to the surface area of the pond at the control elevation. The littoral zone shall be no deeper than 3.5 feet below the design overflow elevation. The treatment volume should not cause the pond level to rise more than 18 inches above the control elevation. Mulching and/or planting is desirable but not required, unless the soils in the proposed littoral zone are not capable of supporting wetland vegetation. In this case mulching will be required. Native vegetation that becomes established in the littoral zone must be maintained as part of the operation permit.

3. Isolated natural wetlands can be used as a wet detention system when not in conflict with environmental or public use considerations.

(a) If the required treatment volume cannot be detained within the limits of the isolated wetland boundaries and range of natural water levels, expansion of the wetland will be allowed when it can be shown that the excavation will not adversely impact the wetland.

(b) The treatment volume cannot adversely impact the wetland so that it fluctuates beyond the range of natural water levels. The available volume is determined based on site-specific conditions and an analysis of the isolated wetland to be used.

(c) Provisions must be made to remove sediment, oils and greases from runoff entering the wetland. This can be accomplished through incorporation of sediment sumps, baffles and dry grassed swales or a combination thereof. Normally, a dry grassed swale system designed for detention of the first one-fourth inch of runoff with an overall depth of no more than 4 inches will satisfy the requirement for prior removal of sediment, oils and greases.

4. The wet detention system's treatment volume shall be discharged in no less than 120 hours (5 days) with no more than one-half the total volume being discharged within the first 60 hours (2.5 days).

5. Due to the detention time required for wet detention systems, only that volume which drains below the overflow elevation within 36 hours may be counted as part of the volume required for water quantity storage under AH II Chapter 3.

6. Concepts and methods for determining design pool requirements and alternatives for wet detention system designs can be found in Appendix B – Wet Detention System Concepts and Alternatives.

History Note: SWFWMD ERP Information Manual Part B, section 5.2.a.1-5.

b. Detention with Effluent Filtration System (Manmade Underdrains).

1. A detention with effluent filtration system shall treat the runoff from the first one inch of rainfall; or as an option for projects or project subunits with drainage areas less than 100 acres, the first one-half inch of runoff. In determining the runoff from one inch of rainfall, the applicant must provide calculations determining runoff from the directly connected impervious and semi-impervious areas separately from any other contributing area.

2. Filtration systems shall have a minimum of 0.5 feet of vertical head between the center line of the perforated pipe and the normal water elevation or the pond bottom of the system. The seasonal high water level must be at least one foot below the center line of the perforated pipe (measured from the lowest point of the perforated pipe), or separated by structural means from the hydraulic contribution of the surrounding water table. The stormwater must pass through a minimum of two feet of the filter material before entering the perforated pipe.

3. Filtration systems shall have pore spaces large enough to provide sufficient flow capacity so that the permeability of the filter is equal to or greater than the surrounding soil. The design shall ensure that the filter medium particles do not move. The filter material shall be of a quality sufficient to satisfy the requirements listed below, but these requirements are not intended to preclude the use of multilayered filters nor the use of materials to increase ion exchange, precipitation or pollutant absorption capacity of the filter. The requirements are:

(a) Washed material meeting FDOT road and bridge specifications for silica sand and quart gravels, or mixtures thereof (less than 1 percent silt, clay and organic matter), unless filter cloth is used which is suitable to retain the silt, clay and organic matter within the filter; calcium carbonate aggregate is not an acceptable substitute;

(b) Uniformity coefficient 1.5 or greater; and

(c) Effective grain size of 0.20 to 0.55 millimeters in diameter.

4. The total detention volume shall again be available within 36 hours.

5. The treatment volume can be counted as part of the storage required for water quantity storage under AH II Chapter 3.

6. Maintenance of filter includes proper disposal of spent filter material.

7. The design of the system must be such that the water velocities and associated flow path through the storage pond do not cause the accumulated pollutants to be flushed out of the treatment pond up to the 25-year, 24-hour design storm.

History Note: SWFWMD ERP Information Manual Part B, section 5.2.b

c. On-line Treatment Systems.

1. An on-line treatment system shall treat the runoff from the first one-inch of rainfall; or as an option for projects or project sub-units with drainage areas less than 100 acres, the first one-half inch of run-off. In determining the runoff from one-inch of rainfall, the applicant must provide calculations determining runoff from the directly connected impervious and semi-impervious areas separately from any other contributing area.

2. Total treatment volume shall again be available within 72 hours, however, only that volume which can again be available within 36 hours may be counted as part of the volume required for water quantity storage under AH II Chapter 3.

3. The design of the system must be such that the water velocities and associated flow path through the storage pond do not cause the accumulated pollutants to be flushed out of the treatment pond up to the 25-year, 24-hour design storm.

History Note: SWFWMD ERP Information Manual Part B, section 5.2.c.

d. Off-line Treatment Systems.

1. Off-line treatment systems shall treat the runoff from the first one-inch of rainfall; or as an option for projects or project sub-units with drainage areas less than 100 acres, the first one-half inch of runoff. In determining the runoff from one-inch of rainfall, the applicant must provide calculations determining run-off from the directly connected impervious and semi-impervious areas separately from any other contributing area.

2. Total treatment volume shall again be available within 72 hours, however, only that volume which can again be available within 36 hours may be counted as part of the volume required for water quantity storage under AH II Chapter 3.

History Note: SWFWMD ERP Information Manual Part B, section 5.2.d

e. Underground Exfiltration Systems.

1. Systems shall be designed for the volumes specified in Section 4.1(d) for off-line treatment systems.

2. Systems must have the capacity to retain the required retention volume without considering discharges.

3. The seasonal high water level must be at least one foot below the bottom of the exfiltration pipe.

4. Systems should not be proposed for projects to be operated by entities other than single owners or entities with full time maintenance staffs.

5. A safety factor of 2.0 or more shall be applied to the exfiltration design to allow for geological uncertainties by dividing the exfiltration rate by the safety factor.

6. Total system required volume shall again be available within 72 hours.

7. Due to the maintenance requirements and life expectancy of exfiltrations systems, the treatment volume required in Section 4.1(d) cannot be counted as part of the storage volumes required under AH II Water Quantity Section 3.3.

8. Exfiltration systems shall comply with the following construction requirements:

- a. Pipe diameter must be a minimum of 12 inches;
- b. Trench width must be a minimum of 3 feet;
- c. Rock material in trenches must be enclosed in filter material; and
- d. Maintenance sumps must be provided in inlets.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Sections 5.7 and 6.5.

f. Discharges to Outstanding Florida Waters.

Projects discharging directly into Outstanding Florida Waters (OFW) shall be required to provide treatment for a volume 50 percent more than required for the selected treatment system (wet detention, detention with effluent filtration, on-line retention or off-line retention).

History Note: SWFWMD ERP Information Manual Part B, section 5.2.e.

g. Where Ambient Water Quality Does Not Meet Standards.

In instances where an applicant is unable to meet water quality standards because existing ambient water quality does not meet standards and the system will contribute to this existing condition, mitigation for water quality impacts can consist of water quality enhancement. In these cases, the applicant must implement mitigation measures that are proposed by or acceptable to the applicant that will cause net improvement of the water quality in the receiving waters for those contributed parameters that do not meet standards.

History Note: SWFWMD ERP Information Manual Part B, section 3.2.4.5.

h. Off-site Treatment Volumes.

Off-site treatment volumes shall be the total runoff from one-inch of rainfall over the contributing off-site area. The runoff from the directly connected impervious and semi-impervious contributing areas shall be determined separately from the runoff from the other contributing areas.

History Note: SWFWMD ERP Information Manual Part B, section 5.2.f.

4.2 Public Supply Wells.

Surface water treatment systems shall not be located closer than 100 feet from public water supply wells.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 5.3

4.3 Sewage Treatment Percolation Ponds.

Above ground pond dikes shall not be located within 200 feet of water bodies or 100 feet of dry retention areas. The applicant may propose specific alternative measures that are equivalent to these criteria in their effectiveness to protect the water resources and adjacent property. The applicant shall provide the District with reasonable assurance based on the plans, calculations and other information specific to the design proposed.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 5.4.

4.4 Solid Waste Facilities.

Surface water management systems for Class I and II solid waste facilities, as defined in Chapter 62-7, F.A.C., shall be designed and constructed to maintain the integrity of the landfill at all times including construction, operation, closure and post closure. Applicants should

consult with District staff prior to submittal of an application to determine the specific requirements which will apply for a particular project.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 5.5.

4.5 Alterations to Existing Public Roadway Projects.

Alterations to existing public roadway projects will be required to treat a volume equal to those specified in Section 4.1 and the contributing area according to the following options:

a. The contributing area(s) to be used in calculating the required treatment volume will be:

1. For off-line treatment systems and on-line treatment systems, including wet-detention, which provide storage of the treatment volume off-line from the primary conveyance path of flood discharges, use the area of new pavement.

2. For all other on-line treatment systems, including wet-detention, use the entire on-site directly connected impervious areas contributing to the system; directly connected impervious areas are those new and existing pavement areas connected to the treatment systems by pavement or pipe that contribute untreated runoff.

b. When alterations involve extreme hardship, in order to provide direct treatment of new project area, the District will consider proposals to satisfy the overall public interest that shall include equivalent treatment of alternate existing pavement areas to achieve the required pollution abatement. For example, existing untreated contributing areas not otherwise required to be included for treatment may be included for treatment by the system in lieu of direct treatment of new project area when the pollution abatement is equivalent and benefits the same receiving waters.

c. Existing treatment capacity being displaced by any roadway project will require additional compensating treatment volume. Additional volume is also required for projects that discharge directly to OFW's. (See Section 4.1.f.)

Subsection 373.413(6), F.S. (2012), also requires that Agencies exercise flexibility in the permitting of stormwater management systems associated with the construction or alteration of systems serving state transportation projects and facilities.

History Note: SWFWMD ERP Information Manual Part B Basis of Review, Section 5.8.

4.6 Water Quality Monitoring.

All non-exempt surface water management systems will be evaluated based on the ability of the system to prevent degradation of receiving waters and its ability to conform to state water quality standards.

History Note: SWFWMD ERP Information Manual Part B Basis of Review, Section 5.9.

4.7 General and Special Conditions Related To Water Quality Monitoring By Permittees.

- a. If the applicant utilizes design criteria found in this chapter, monitoring will not be required.
- b. Monitoring shall be required when the applicant proposes design criteria not found in this chapter, and does not have specific test data or other data to support that state water quality standards will be met.
- c. Monitoring may be required in cases where there may be a real and immediate concern regarding degradation of quality in the receiving waters, regardless of the pollutant removal efficiency of the drainage system.
- d. The reason for the monitoring requirement will be stated in each permit for which water quality monitoring is required, along with the monitoring schedule and the parameters of interest. Samples will be collected at discharge locations unless other locations are identified in the monitoring schedule. Monitoring schedules will require the periodic collection of samples. Permittees will also be required to collect samples during storm events, provide the rate of discharge and total discharge quantities at the time of sample collection, if necessary to ensure that state water quality standards will be met.
- e. Permits for projects not requiring water quality monitoring at the time of permit issuance will include a statement that water quality monitoring will be required in the future if necessary to ensure that state water quality standards are being met. This should not be construed as an indication that the District is contemplating the implementation of a program of intensive water quality monitoring by all permittees.

History Note: SWFWMD ERP Information Manual Part B Basis of Review, Sections 5.10, 5.11 and 5.13.

4.8 Compensating Stormwater Treatment.

Occasionally, applicants find that it is impractical to construct a stormwater management system to capture the runoff from a portion of the project site due to on-site conditions such as extreme physical limitations, availability of right-of-way, or maintenance access. Two methods have been developed to compensate for the lack of treatment for a portion of a project. The first method is to treat the runoff that is captured to a greater extent than required by rule (i.e., "overtreatment"). The second method is to provide treatment for an off-site area which currently is not being treated (i.e., "off-site compensation"). Each method is designed to furnish the same level of treatment as if the runoff from the entire project site was captured and treated in accordance with the provisions of this Volume.

Either of these methods will only be allowed as a last resort and the applicant is strongly encouraged to schedule a pre-application conference with District staff to discuss the project if

these alternatives are being considered. Other rule criteria, such as peak discharge attenuation, will still have to be met if the applicant utilizes these methods. Each alternative is described in more detail in the following sections.

History Note: NFWMD AH II section 2.11.

4.8.1 Overtreatment.

Overtreatment means to treat the runoff from the project area that does flow to a treatment system to a higher level than the rule requires, to make up for the lack of treatment for a portion of the project. The average treatment efficiency of the areas treated and the areas not treated must meet the pollutant removal goals of Chapter 62-40, F.A.C., (i.e., 80% removal for discharges to Class III waters and 95% removal for systems which discharge to OFWs). To meet these goals, the area not being treated generally must be small (less than 10%) in relation to the area which is captured and treated. Staff can aid in determining the proper level of overtreatment for a particular situation.

History Note: NFWMD AH II Section 2.11.1.

4.8.2 Off-site Compensation.

Off-site compensation means to provide treatment to compensate for the lack of treatment for portions of the proposed project. The following conditions must be met when utilizing off-site compensation:

- (a) The off-site area must be in the same watershed and benefit the same receiving water body as the proposed project, and should be in the closest vicinity practicable to the location of those untreated stormwater discharge(s) requiring compensating treatment; and
- (b) The applicant shall use modeling or other data analysis techniques that provide reasonable assurance that the compensating treatment system removes at least the same amount of stormwater pollution loading as was estimated from the untreated project area.

History Note: NFWMD AH II Section 2.11.2.

PART V –CONSTRUCTION DESIGN REQUIREMENTS

5.0 Design Criteria.

The design criteria and construction requirements applicable within this District for stormwater management system discharge or control structures, retention and detention areas and other system features are set forth in this chapter. To assist the applicant, additional reference materials and figures useful in designing stormwater management systems appear in the Appendices and should be consulted.

History Note: New

5.1 Discharge Structures.

a. The construction design for all surface water systems shall be adequate to meet all design criteria and performance standards referred to in this rule. Provision shall be made for the controlled release of water volumes in excess of that caused by the design storm event to ensure adequate performance of the system and its continued safe operation. Construction designs shall include adequate provisions to allow operation and maintenance activities and to prevent unauthorized operation of operable structures.

b. All design discharges shall be made through structural discharge facilities. Discharge structures shall be fixed so that discharge cannot be made below the control elevation, except that emergency operation devices may be designed and installed with secure locking mechanisms.

c. Non-operable discharge structures shall not be constructed so that they are operable.

d. Discharge structures shall include gratings for safety and maintenance purposes. The use of trash collection screens is desirable.

e. Discharge structures for water quality systems shall include a "baffle" system to encourage discharge from the center of the water column rather than the top or bottom. Discharge structures from areas with greater than 50 percent impervious and semi-impervious area or from systems with inlets in paved areas shall include a baffle, skimmer, or other mechanism suitable for preventing oil and grease from discharging from detention and on-line treatment systems.

f. Direct discharges, such as through culverts, stormdrains, weir structures, etc., will be allowed to receiving waters which by virtue of their large capacity, configuration, etc. are easily able to absorb concentrated discharges. Examples of such receiving waters include existing storm sewer systems and man-made ditches, canals and lakes.

g. Indirect discharges, such as overflow and spreader swales, are required where the receiving water or its adjacent supporting ecosystem might be degraded by a direct

discharge. The discharge structure must discharge into the overflow, spreader swale, etc. which in turn releases the water to the actual receiving water. Affected receiving waters include natural streams, lakes, marshes, isolated wetlands and land naturally receiving overland sheet flow.

h. Pumped systems will only be allowed for single owner or governmental agency operation entities, unless perpetual operation ability can be guaranteed.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.1.

5.2 Control Devices/Bleed-Down Mechanisms for Detention Systems.

a. When not in conflict with meeting the District's pre-/post-peak discharge requirement or a more restrictive local government discharge requirement, gravity control devices normally shall be designed to discharge one-half of the detention volume required by Chapter 4, within 24 hours. Devices incorporating dimensions smaller than six square inches of cross sectional area or two inches minimum dimension or less than 20 degrees for "V" notches shall include a device to eliminate clogging. Such devices include baffles, grates, pipe elbows, etc.

b. Gravity control devices for wet detention water treatment systems as specified in Chapter 5 are required to be designed to meet the bleed-down times specified therein. Devices incorporating dimensions smaller than those indicated in a. above, must include a device to eliminate clogging. Such devices include baffles, grates, pipe elbows, etc.

c. Wet detention systems designed for both water treatment (quality) and attenuation of the design storm (quantity) must incorporate the requirements of a. and b. above.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.2.

5.3 Maintenance Considerations. The design of retention areas shall incorporate consideration of sediment removal, regular maintenance and vegetation harvesting procedures.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.3.

5.4 Retention and Detention Areas.

5.4.1 Dimensional Criteria (as measured at or from the control elevation).

a. Width - Wet detention water quality treatment systems shall be designed with a 100 foot minimum width for linear areas in excess of 200 feet in length. Area and width requirements will be waived for projects to be operated by single owner entities, or entities with full time maintenance staffs with a particular interest in maintaining the area, e.g., golf courses.

Treatment areas not meeting the above width to length ratio will be approved if the permittee can demonstrate that the design of the system will maximize circulation by location of inflow and outflow points.

b. Depth - The detention or retention area shall not be excavated to a depth that breaches an aquitard such that it would allow for lesser quality water to pass, either way, between the two systems. In those geographical areas of the District where there is not an aquitard present, the depth of the pond shall not be excavated to within two (2) feet of the underlying limestone which is part of a drinking water aquifer.

c. Side slopes – for purposes of public safety, water quality treatment and maintenance, all retention or detention areas should have stabilized side slopes no steeper than 4:1 (horizontal:vertical) out to a depth of two feet below the control elevation. Except as provided for in paragraph 6.4.1(d), constructed side slopes steeper than 3.5:1 (horizontal:vertical) shall be considered a substantial deviation from the permitted design.

d. For purposes of public safety, side slopes designed or permitted steeper than 4:1 will require a six foot chain link fence or other protection sufficient to prevent accidental incursion into the retention or detention area. In determining the sufficiency of other protection measures, consideration shall be given to the depth and morphometry of the detention or retention area, surrounding land uses, degree of public access, and likelihood of accidental incursion.

e. For wet detention systems, the bottom elevation of the pond must be at least one foot below the control elevation.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.4.1 for (a) – (d); Section 1.7.4 for (e).

5.4.2 Maintenance Access.

Perimeter maintenance and operation easements, with a minimum width of 20 feet and slopes no steeper than 4:1 (horizontal: vertical), should be provided landward of the control elevation water line. Widths less than 20 feet are allowed when it can be demonstrated that equipment can enter and perform the necessary maintenance for the system.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.4.2.

5.5 Exfiltration Systems Dimensional Criteria.

Exfiltration systems shall comply with the following construction requirements:

- a. Pipe diameter must be a minimum of 12 inches;
- b. Trench width must be a minimum of 3 feet;

- c. Rock material in trenches must be enclosed in filter material; and
- d. Maintenance sumps must be provided in inlets.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.5.

5.6 Management of Runoff from Impervious and Semi-Impervious Areas.

Runoff shall be discharged from impervious and semi-impervious surfaces into retention areas, or through detention devices, filtering and cleansing devices, or subjected to some type of Best Management Practice (BMP) prior to discharge from the project site. For projects, which include substantial paved areas, such as shopping centers, large highway intersections with frequent stopped traffic, and high density developments, provisions shall be made for the removal of oil, grease and sediment from storm water discharges.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.6.

5.7 Stagnant Water Conditions.

Configurations which create stagnant water conditions, such as dead end canals, are prohibited, regardless of the type of development.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.7.

5.8 Sediment Sumps.

Sediment sumps shall comply with the following:

- a. Sumps shall remove a particle size of 0.1 mm in diameter (approximately a No. 100 sieve size) unless it can be shown another grain size is more appropriate for the site.
- b. Sumps shall be designed for an inflow rate equal to the design peak flow rate of the project's internal storm water system.
- c. A maintenance schedule for sediment and vegetation removal must be included.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.8.

5.9 Dam Safety.

All dams must be designed, constructed, operated and maintained consistent with accepted engineering and dam safety practices as applied to local conditions, considering such factors as type of materials, type of soils and degree of compaction, hydrologic capacity, construction

techniques and hazard rating. A document that provides useful information for this purpose is *Design of Small Dams*, U.S. Department of the Interior, Bureau of Reclamation, Third Edition, 2006.

History Note: Adapted from NFWMD AH I Section 8.4.7.

5.10 Rural or Minor Residential Subdivisions.

a. Rural or minor residential subdivisions typically are designed to have large multi-acre lots and minimal roadways that, together, result in a relatively small amount of additional impervious or semi-impervious surfaces compared to pre-developed conditions. Rural or minor residential subdivisions that are designed in accordance with the following parameters will be considered to not cause significant adverse impacts to occur individually or cumulatively and will meet the applicable water quality and water quantity design criteria for permit issuance:

(1) The proposed activities will occur in, on or over less than 100 square feet of wetlands or other surface waters. Road or driveway crossings of ditches constructed in uplands will not be counted against the 100 square foot limit.

(2) The activities will not utilize pumps for storm water management.

(3) The activities will not utilize storm drainage facilities larger than one 24-inch diameter pipe, or its equivalent.

(4) Discharges from the site will meet state water quality standards.

(5) The proposed building floors will be above the 100 year flood elevation.

(6) The surface water management system can be effectively operated and maintained.

(7) Roadways within the subdivision will consist of paved or unpaved stabilized roads with an unyielding subgrade.

(8) The drainage system will not act in a manner that would divert and channelize large areas of overland sheet flow, thereby creating point source discharges that will adversely affect wetlands, or areas beyond the applicant's perpetual control.

(9) Point discharges will not exceed the capacity of receiving waters.

(10) All terminal discharge structures are designed to withstand the 25-year, 24-hour post-development discharge without functional failure.

(11) The proposed post-development impervious and semi-impervious surfaces will not exceed a five percent (5%) increase over pre-developed conditions.

(12) Proposed or projected construction will maintain a minimum 75 foot vegetated

buffer, which includes a 25 foot perpetually undisturbed buffer upland of all wetlands and other surface waters. Only the 25 foot perpetually undisturbed buffer will be required adjacent to an isolated wetland entirely located within an individual residential lot.

(13) Proposed or projected construction will maintain a minimum 75 foot buffer adjacent to all project boundaries.

b. The applicant's demonstration of compliance with this subsection shall include provision of a typical lot layout showing proposed driveways, buildings, and other impervious and semi-impervious areas and the anticipated percentage of impervious and semi-impervious surfaces resulting from projected construction on individual residential lots.

c. The boundaries of the surface water management system, wetlands, surface waters and buffers shall be recorded in plats or easements and included in any declaration of covenants, conditions, easements and restrictions and shall be identified in all sales contracts by the developer. These recorded documents shall be perpetual and applicable to all future sales of property within the development. Language shall also be contained in the recorded documents notifying all individual lot owners that permits are required if any of the following items are proposed:

- (1) Alteration to the surface water management system; or
- (2) Encroachment into the wetlands, wetland buffers, or adjacent off-site property line buffers.

History note: Transferred from 40D-40.301(1) and (2), F.A.C.

5.11 Sensitive Karst Areas.

"Karst" is a geologic term used to describe areas where landscapes have been affected by the dissolution of limestone or dolostone, including areas where the formation of sinkholes is relatively common. In parts of the District, limestone (or dolostone) that makes up or comprises the Floridan Aquifer System occurs at or near the land surface. Sediments overlying the limestone can be highly permeable. Due to its chemical composition, limestone is susceptible to dissolution when it interacts with slightly acidic water. "Sensitive karst areas" reflect areas with hydrogeologic and geologic characteristics relatively more conducive to potential contamination of the Floridan Aquifer System from surface pollutant sources. The formation of karst-related features, such as sinkholes, is also more likely to occur in these areas.

Especially In sensitive karst areas, stormwater management systems must be designed and constructed to prevent direct discharge of untreated stormwater into the Floridan Aquifer System. Systems also must be designed and constructed in a manner that avoids breaching an aquitard and such that construction excavation will not allow direct mixing of untreated water between surface waters and the Floridan Aquifer System. The system shall also be designed to prevent the formation of solution pipes or other types of karst features in any known sensitive karst area. Test borings located within the footprint of a proposed stormwater management pond must be plugged in a manner to prevent mixing of surface and ground waters.

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As provided in AH II Section 5.4.1.b, in areas where karst conditions are present, the detention or retention area shall not be excavated to a depth that breaches an aquitard such that it would allow for lesser quality water to pass, either way, between the two systems.

Figures depicting conditions that may occur when retention or detention ponds are constructed in sensitive karst areas appear in Appendix C.

History Note: Adapted from NFWMD AH II sections 17.1 and 17.3; SWFWMD ERP Information Manual Part B, Basis of Review, Section 6.4.1.b.

PART VI – DESIGN INFORMATION

6.0 Design Criteria.

The design criteria set forth in this section are applicable within this District.

History Note: New

6.1 Antecedent Conditions.

Within this District, the antecedent condition will be the normal average wet season (AMC II).

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.1.

6.2 Rainfall Volume.

The rainfall isohyetal maps in AH II APPENDIX A will be used to determine rainfall amounts.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.2; Part D Project Design Aids.

6.3 Rainfall Distribution.

The Natural Resource Conservation Service Type II Florida Modified rainfall distribution will be used unless the applicant demonstrates that a different distribution better characterizes the actual rainfall distribution based on rainfall record.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.3.

6.4 Open Surface Storage.

If open surface storage is to be considered in the review, the applicant must submit stage-storage computations. If open surface storage plus discharge is to be considered, the stage discharge computations will also be submitted. Actual rather than allowable discharges shall be used in routing. Discharges will be based on the tail water resulting from the normal seasonal high water elevation of the receiving waters. For extreme events, such as the 100-year frequency, discharge will be based on the tail water resulting from a 100-year flood on the receiving waters.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.4.1.

6.5 Ground Surface Infiltration.

Ground surface infiltration will be reviewed on the basis of commonly accepted procedures such as those of Soil Conservation Service (see U.S. Department of Agriculture, Soil Conservation Service Technical Paper No. 149, "A Method for Estimating Volume and rate of Runoff in Small Watersheds" (1973), and U.S. Department of Agriculture, Soil Conservation Service Technical Release No. 55, "Urban Hydrology for Small Watersheds" (1975); or Rational Method (see State of Florida Department of Transportation, "Drainage Manual" (1987); or standard civil engineering textbooks), unless test data are submitted to justify other procedures.

Additional, more current references and design aids are listed in Appendix D.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.5.1.

6.6 Subsurface Exfiltration.

Subsurface exfiltration will be reviewed only on the basis of representative or actual test data submitted by the applicant. Tests shall be consistent as to elevation, location, soils, etc. with the system design to which the test data will be applied.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.5.2.

6.7 Runoff.

The usual methods of computation of runoff are as follows:

- a. Rainfall minus losses and storage.
- b. Soil Conservation Service (see U.S. Department of Agriculture, Soil Conservation Service, "National Engineering Handbook, Section 4, Hydrology." Additional, more current reference sources and design aids can be found in Appendix D.)
- c. Rational method, for systems serving projects of less than 10 acres total contributing area (see State of Florida Department of Transportation, "Drainage Manual" Volume 2A 1987; or standard civil engineering texts. Additional, more current references and design aids are listed in Appendix D).
- d. Other alternative methods and criteria proposed by the applicant that are functionally equivalent to the criteria in District rules. The applicant shall provide the District with reasonable assurance of such equivalency based on the submitted plans, calculations and other information.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.6.

6.8 Allowable Discharges.

Peak discharge, for purposes of meeting maximum allowable discharges, is computed as the maximum average discharge over a time period equal to the time of concentration of the contributory area.

History Note: SWFWMD ERP Information Manual Part B, Basis of Review, Section 7.8.1

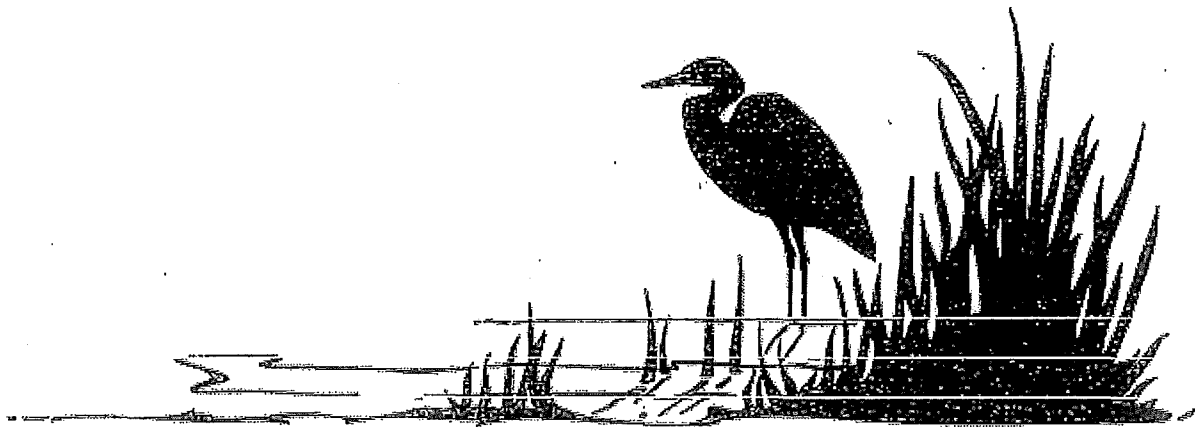
APPENDICES

- APPENDIX A – Part D of SWFWMD ERP Information Manual Rainfall Maps
- APPENDIX B – Concepts and Methods for Determining Design Pool Requirements and Alternatives for Wet Detention Systems
- APPENDIX C – Figures Relating to Water Quality Provisions, Water Quantity Provisions and Retention Systems Within Sensitive Karst Areas
- APPENDIX D – References and Design Aids for Designing Effective Stormwater Treatment Systems
- APPENDIX E – References and Website Links for Agricultural Best Management Practices and Materials
- APPENDIX F – Addresses and contacts for District offices
- APPENDIX G - Airport Best Management Practices Manual (proposed)

Southwest Florida Water Management District

PART D
PROJECT DESIGN AIDS

ENVIRONMENTAL RESOURCE PERMITTING
INFORMATION MANUAL



MANAGEMENT AND STORAGE OF SURFACE WATERS

JULY 1996

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PART D - PROJECT DESIGN AIDS

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1. INTRODUCTION

The Southwest Florida Water Management District has developed "Part D, Project Design Aids" to provide consistency in the parameters used by District Staff in their review for environmental resource permitting of surface water management systems.

The design aids were selected solely on their ability to support the intent of the requirements in the environmental resource permitting rules for management and storage of surface waters and to identify some common and useful techniques for evaluating regulatory aspects of surface water management systems. The use of these design aids for purposes other than those stated in this manual may not be within the realm of their intended use and could result in a sub-standard design in some circumstances. It is therefore, incumbent upon the individual designer to exercise sound engineering judgement in the utilization of the information presented to ensure the overall integrity of the system and regulatory compliance.

2. Rainfall

A. Rainfall Duration

The Southwest Florida Water Management District is utilizing a 24-hour storm event as the standard storm duration for design and analysis purposes for water quantity permitting evaluation of surface water management systems.

B. Determination of Rainfall Depths

The Southwest Florida Water Management District has developed rainfall maps for a 24-hour storm duration for the 2-year, 2.33-year, 5-year, 10-year, 25-year, 50-year, and 100-year return periods, as shown in FIGURES D-1 through D-7. These rainfall maps will be utilized to determine a depth of rainfall in inches for a specific return period. This depth will be used for design and analysis purposes for evaluation of surface water and stormwater management systems.

C. Procedure for Determination of the Appropriate Rainfall Amount

1. The approximate location of the project site is to be located on the appropriate rainfall frequency map.
2. For projects located on an isohyet use the rainfall amount for that line.
3. For projects east of the most eastern isohyet use that eastern most isohyet as the rainfall amount.
4. For projects between two isohyet the rainfall amount is a straight line interpolation between the two isohyet. The next higher isohyet line may be used rather than interpolating.
5. For projects west of the most western isohyet use that western most isohyet as the rainfall amount.

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

TWENTY FOUR HOUR
TWO YEAR
RETURN PERIOD
RAINFALL MAP

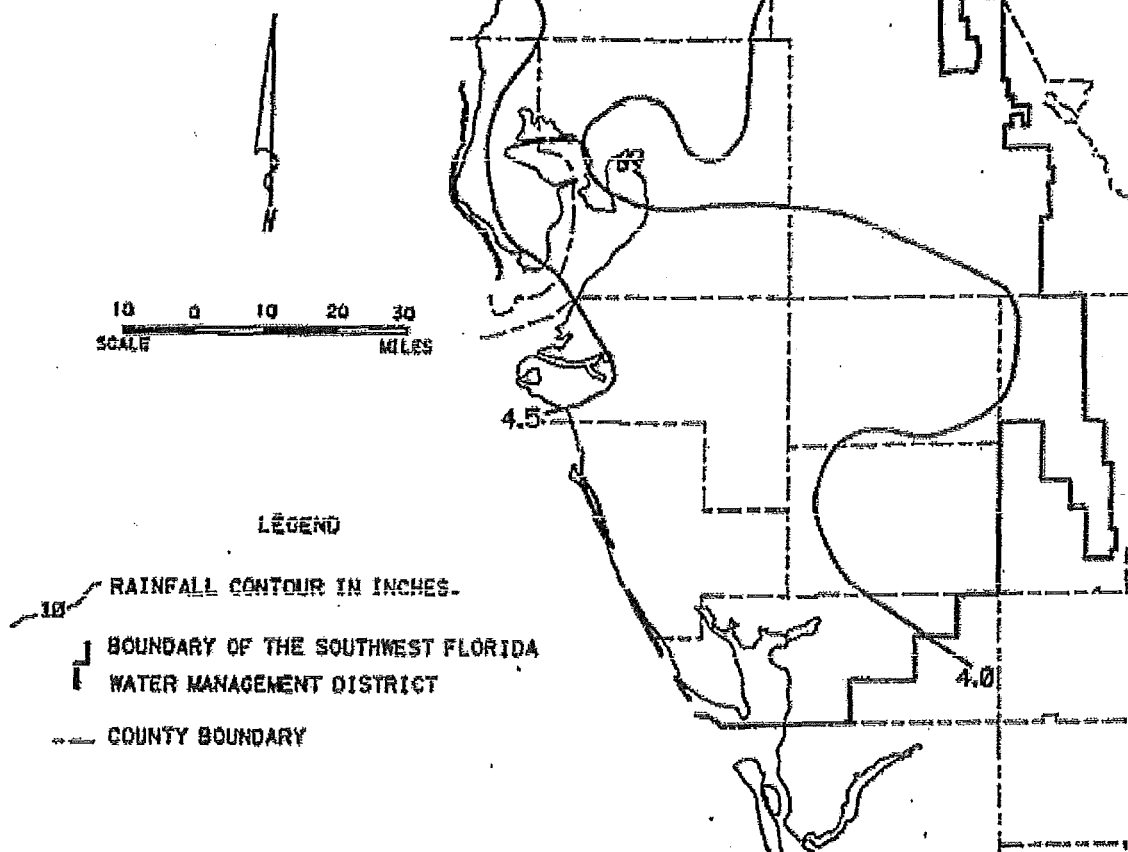


FIGURE D-1

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

TWENTY FOUR HOUR
MEAN ANNUAL (2.33-YEARS)
RETURN PERIOD
RAINFALL MAP

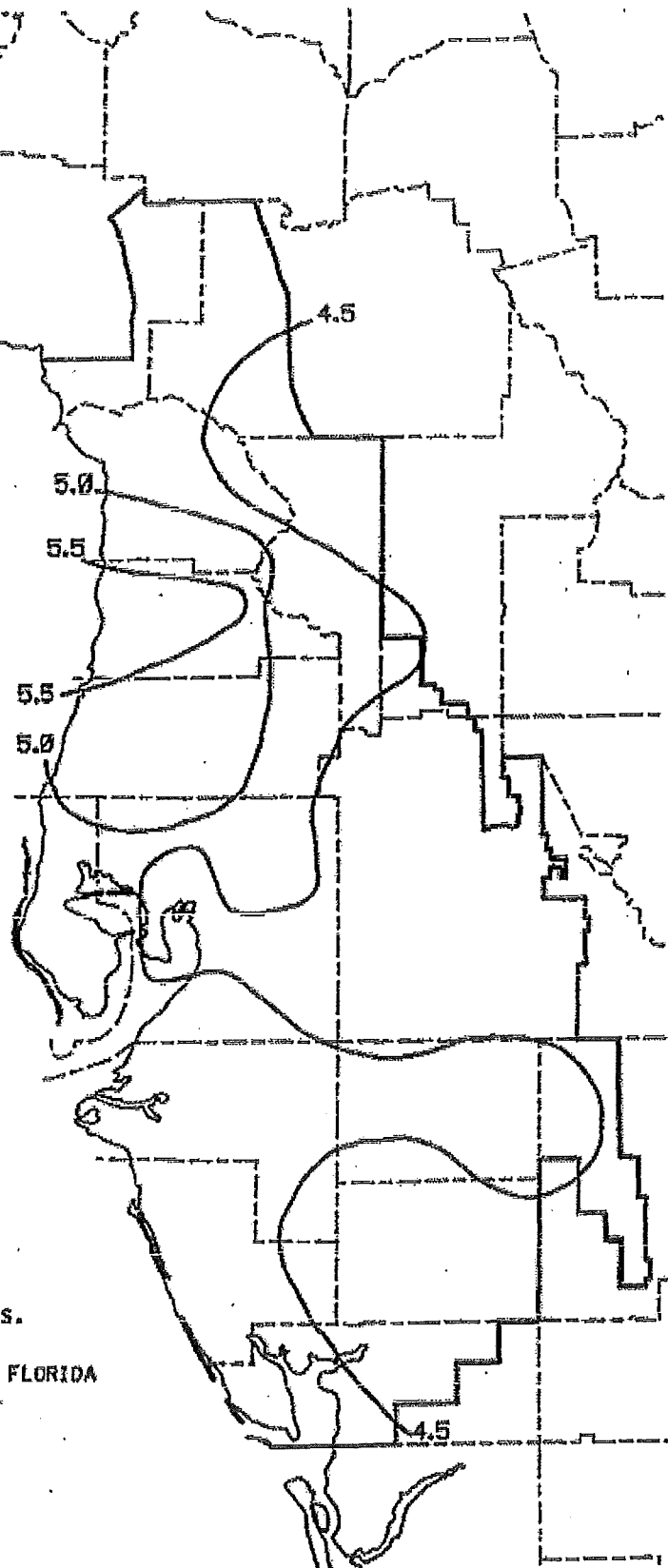
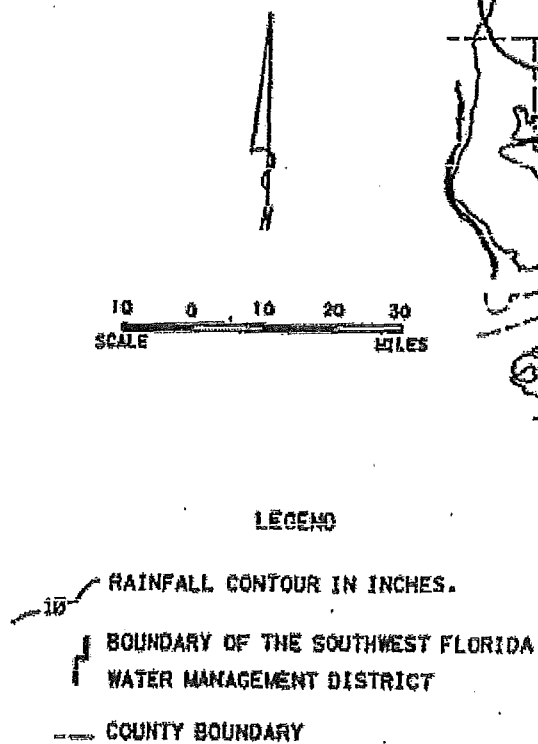


FIGURE D-2

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

TWENTY FOUR HOUR
FIVE YEAR
RETURN PERIOD
RAINFALL MAP

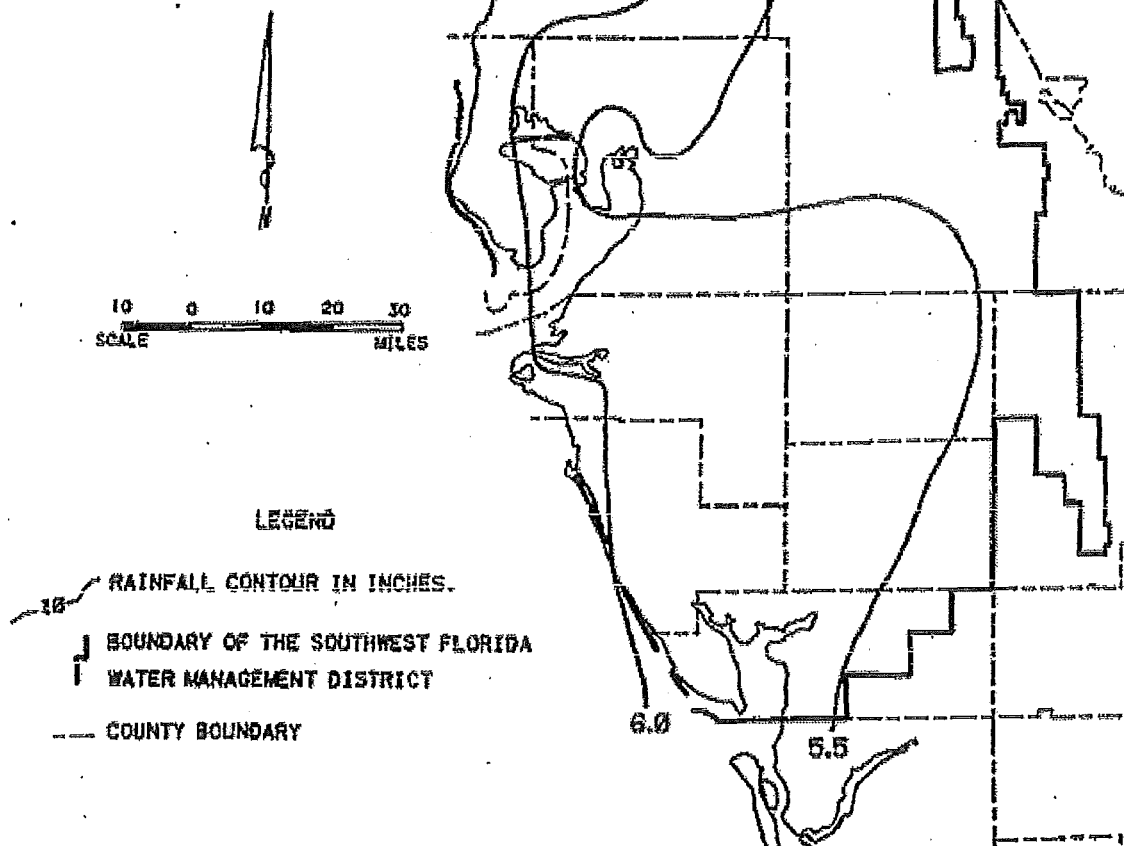


FIGURE D-3

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

TWENTY FOUR HOUR
TEN YEAR
RETURN PERIOD
RAINFALL MAP

10 0 10 20 30
SCALE MILES

LEGEND

- RAINFALL CONTOUR IN INCHES.
- BOUNDARY OF THE SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT
- COUNTY BOUNDARY

FIGURE D-4

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

TWENTY FOUR HOUR
TWENTY FIVE YEAR
RETURN PERIOD
RAINFALL MAP

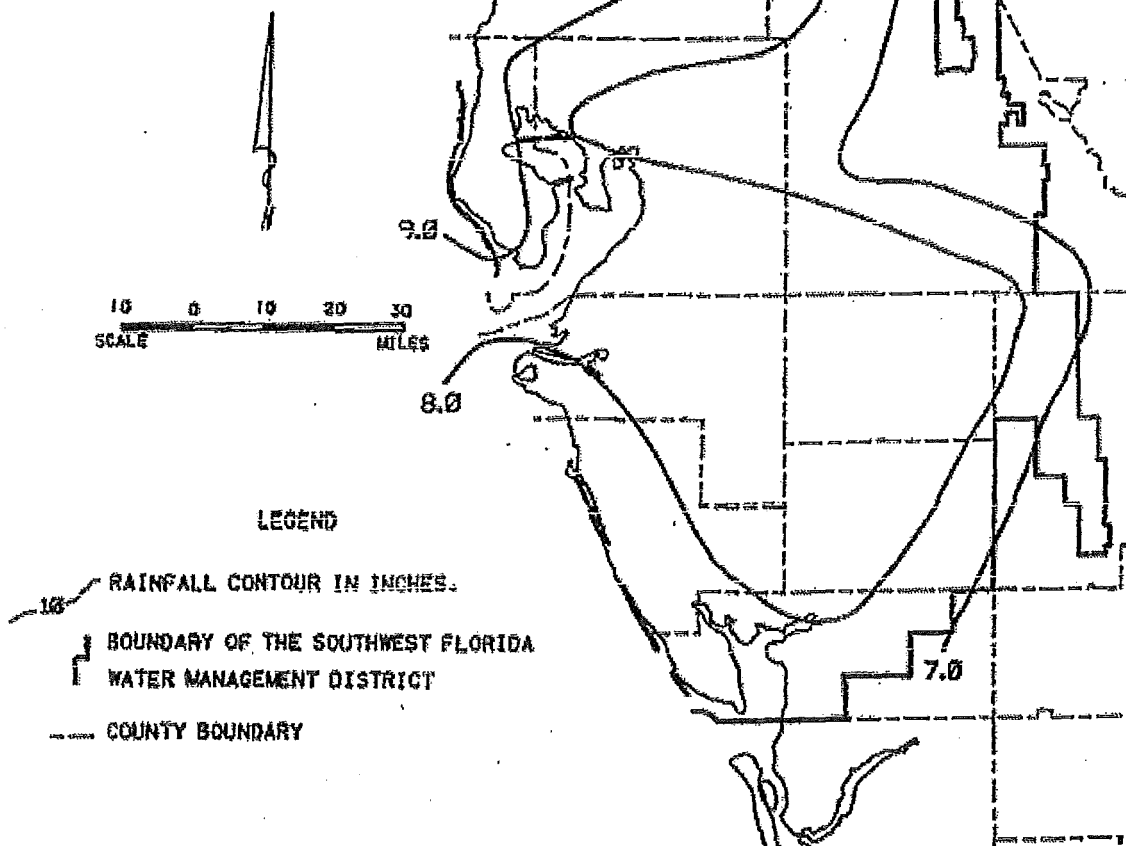
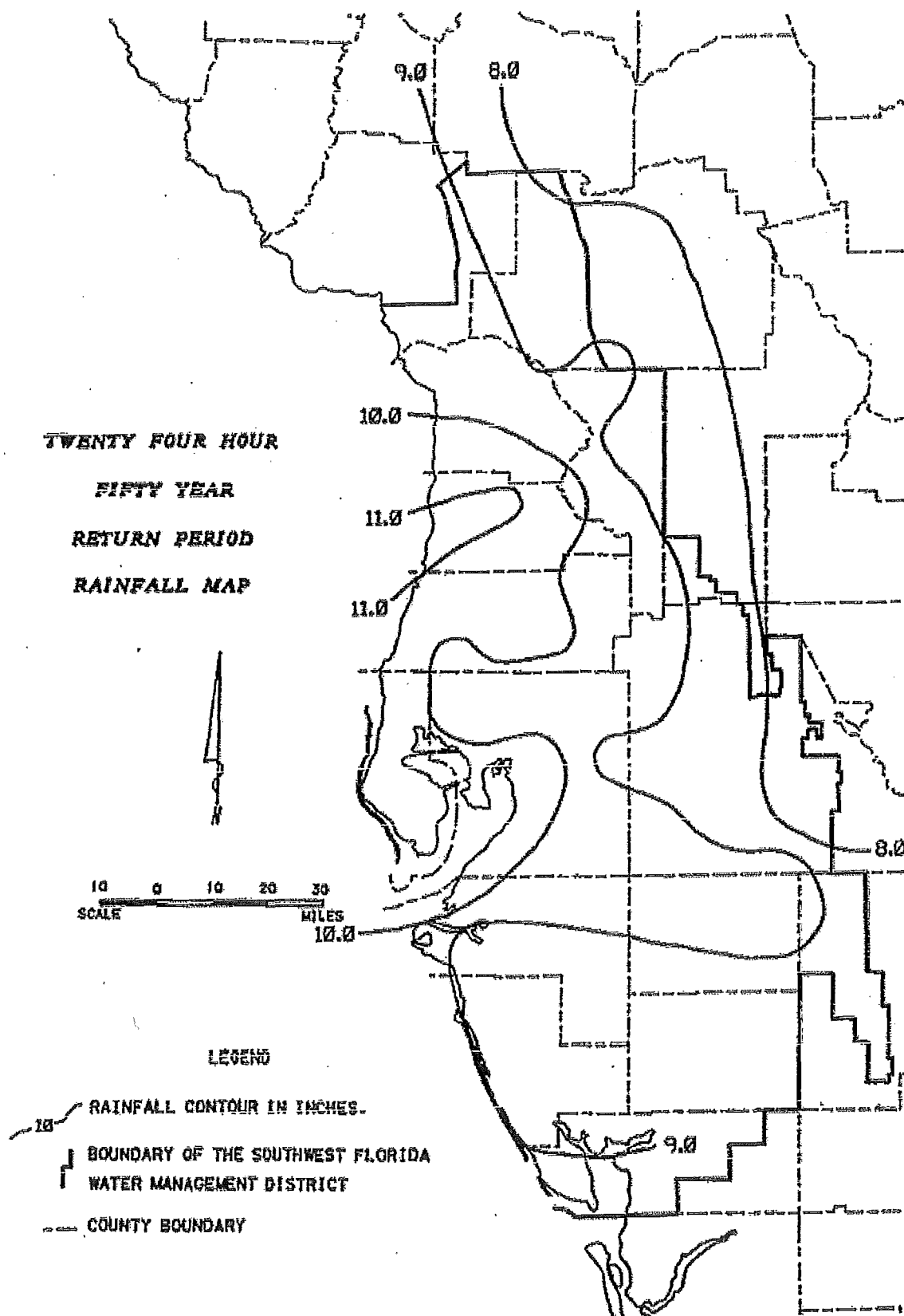


FIGURE D-5

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

**TWENTY FOUR HOUR
FIFTY YEAR
RETURN PERIOD
RAINFALL MAP**



LEGEND

- RAINFALL CONTOUR IN INCHES.
- BOUNDARY OF THE SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT
- COUNTY BOUNDARY

FIGURE D-6

SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

TWENTY FOUR HOUR
ONE HUNDRED YEAR
RETURN PERIOD
RAINFALL MAP

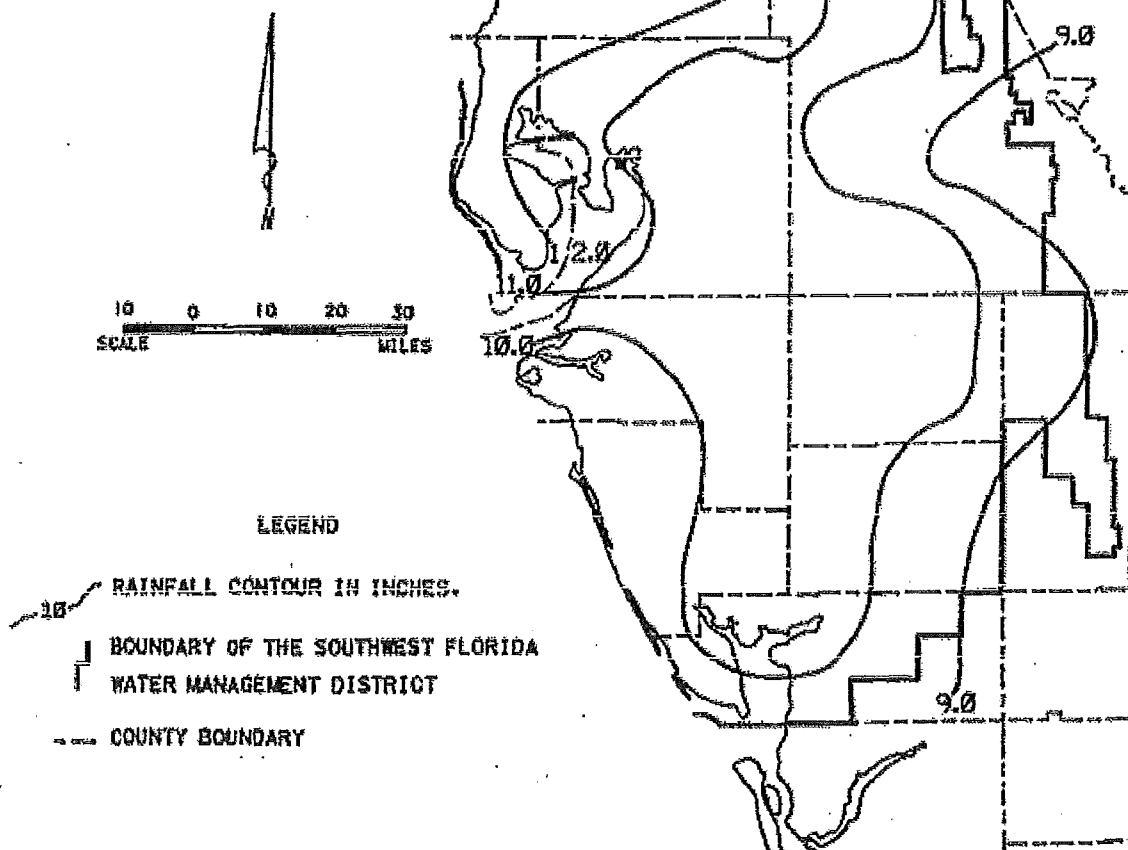


FIGURE D-7

D. Rainfall Distribution

The USDA-Natural Resources Conservation Service [formerly Soil Conservation Service (SCS)] Type II Florida Modified Rainfall distribution shown in **TABLE D-1** is recommended for use in the analysis of the water quantity portion of the rule. Other distributions which produce similar results may also be used if appropriate.

TABLE D-1
RAINFALL RATIOS (ACCUMULATED 24-HOUR TOTAL)

<u>TIME (HR.)</u>	<u>SCS TYPE II FL. MODIFIED</u>
0.0	.000
0.5	.006
1.0	.012
1.5	.019
2.0	.025
2.5	.032
3.0	.039
3.5	.047
4.0	.054
4.5	.062
5.0	.071
5.5	.080
6.0	.089
6.5	.099
7.0	.110
7.5	.122
8.0	.134
8.5	.148
9.0	.164
9.5	.181
10.0	.201
10.5	.226
11.0	.258
11.5	.308
12.0	.607
12.5	.719
13.0	.757
13.5	.785
14.0	.807
14.5	.826
15.0	.842
15.5	.857
16.0	.870
16.5	.882
17.0	.893
17.5	.904
18.0	.913
18.5	.923
19.0	.931
19.5	.940
20.0	.948
20.5	.955
21.0	.962
21.5	.969
22.0	.976
22.5	.983
23.0	.989
23.5	.995
24.0	1.000

APPENDIX B

Concepts and Methods for Determining Design Pool Requirements And Alternatives for Wet Detention Systems

(Taken from District publication, *Three Design Alternatives for Stormwater Detention Ponds*,
June 1997, Appendix A)

**SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT
RESOURCE REGULATION
TECHNICAL PROCEDURE FOR CONSERVATION WET DETENTION**

The design guidelines for the Conservation Wet Detention criteria (14-day residence time) are included here for the convenience of anyone wishing to use them. They include the wet detention design pool guidelines that provided the best water quality treatment during this study. The following section is adapted from the original technical procedure developed by SWFWMD's Technical Services Staff in August 1990. The original draft included three alternatives, but only the third alternative, the conservation wet detention design, is included here since those guidelines were the ones used to construct the pond during the third year of this study (1994). Examples for making calculations for the conservation wet detention design are also provided.

This procedure provides interim guidelines regarding concepts and methods for determining design pool¹ requirements and alternatives for wet detention systems used for stormwater quality treatment..

BACKGROUND: Sections 2.0, 3.2.2, 3.2.3 and 3.2.4 in the Basis of Review (BOR) for the management and storage of surface water (MSSW)(Reference 1), contain guidelines for wet detention systems to provide water quality treatment using a design pool in association with water tolerant vegetation. If adequate residence time is provided, pollutants can be removed through settling, adsorption to soils and uptake by aquatic biota.

The explanation of a wet detention system in section 2.25 of the BOR includes a requirement that, "...The bottom elevation of the pond must be at least one foot below the control elevation." The intent of this requirement is to maintain a permanent wet pool which supports residual aquatic biota, dilutes influent stormwater runoff and extends the residence time of water passing through the system.

Design guidelines for wet detention systems in section 3.2.2.2 require that wet detention pond discharge structures normally be designed with a gravity drawdown control device (bleeder). The bleeder allows no more than one-half of the detained treatment volume, stored between the overflow elevation down to seasonal high water level (SHWL) or control elevation, to discharge within the first 60 hours. The Conservation Wet Detention criteria changes this "bleeddown" time to 24 hours. Pool volume below the control elevation that intermixes with the SHWL is the permanent wet pool.

¹ Design pool = treatment volume + permanent wet pool volume.

CONSERVATION WET DETENTION: The following criteria provide acceptable alternative methods of achieving design pool and gravity discharge configuration when it is justified to provide all or part of the treatment volume below SHWL or control elevation, without design pool bleed down². If all other criteria are in compliance with the BOR, monitoring will normally not be required.

- a) In the interest of water conservation, discharge devices below SHWL shall be avoided; and
- b) Design pool volume below the control elevation³ to eight feet depth must be equal to one inch of runoff plus the calculated volume based on average residence time of 14 days and average total rainfall during the wet season (122 days, June through September); and
- c) The minimum design pool volume below the control elevation to eight feet depth must be no less than 1.667 inches of runoff from the contributing area; and
- d) Systems discharging directly into Outstanding Florida Waters (OFW) shall provide treatment and permanent wet pool volume 50 percent more than required for systems discharging to other receiving waters; and
- e) The gravity overflow weir shall be multi-stage, first having a "v"-notch⁴ or other equivalent drawdown control device sized to discharge one-half inch of detention runoff from the contributing area in 24 hours with ten inches maximum head (refer to Figure 1); and having a broad crested weir for higher discharges, including the 25 year, 24 hour event; and
- f) The control elevation ("v"-notch invert) shall be above SHWL in the pond and above wet season tailwater in the receiving water, but no higher than two feet above SHWL; and
- g) For gravity discharge systems with treatment volume below SHWL, credit for water quantity (discharge attenuation) storage may be allowed above control elevation and SHWL, if the "v"-notch meets the requirements of 3) e) and BOR Section 3.2.4.2; and

² Please refer to Clarification Memo No. SWP - 51 for further discussion of circumstances when wet detention systems may justify not using a bleeder.

³ Longer residence time associated with the design pool for a wet detention system without a bleeder is presumed to offset the benefits of extended detention drawdown of treatment volume by a bleeder.

⁴ The "v"-notch weir sized as stated creates a minimum pond area and fluctuation to enhance surface aeration, circulation and mixing in the design pool. The minimum pond area is equivalent to five percent of the contributing area, as recommended by reference 2.

- h) At least 35 percent of the pond bottom, based on area at control elevation, must extend below SHWL to help sustain the required littoral area; and the 35 percent littoral area shall extend two feet maximum below the control elevation; and
- i) Wet detention systems shall be specifically designed to maximize circulation, mixing and residence time of inflow within the design pool by means such as: maximum separation of inflow and outflow points, locating inflow inverts below the control elevation, use of multi-cell ponds or flow baffles and other locally effective means to avoid "dead" storage areas.

AGRICULTURAL EXAMPLE CALCULATION OF WET DETENTION DESIGN POOL VOLUME

Given: A citrus grove project near Arcadia, Florida; Project area = drainage area = 320 Acres; Composite Rational runoff coefficient = 0.30; Discharge to Class III waters from a wet detention system.

- Required:**
1. Calculate the treatment volume; and
 2. Calculate the permanent wet pool volume to be retained below the control elevation to eight feet depth. It must be the greater of: a) the volume calculated to provide an average residence time of 14 days based on average total wet season rainfall of 31.04 inches; or, b) the volume produced by 0.667 inches of runoff from the contributing area; and
 3. Calculate the average minimum pond area.

1. Calculate the treatment volume (Q) as one inch of runoff -

$$\begin{aligned} (Q) &= (320 \text{ Ac.}) (1 \text{ inch}) (1 \text{ ft./12 in.}) \\ &= 26.67 \text{ Ac. - ft. (AF)} \end{aligned}$$

2. Calculate the permanent wet pool volume (V_R) -

- a) Based on 14 day residence volume (V_R) -

$$(V_R) = (A) (C) (P) (R) (1 \text{ ft./12 in.})$$

Where, (A) = Project area = drainage area = 320 Ac
 (C) = Composite Rational runoff coefficient = 0.30
 (P) = Historic average wet season rainfall rate for
 (R) = Residence time = 14 days

$$(V_R) = (320) (0.30) (31.04/122) (14) (1/12)$$

$$= 28.50 \text{ AF}$$

NOTE: Refer to Figure 2 for graphic solution of 14 day residence volumes for various project types and sizes.

b) As 0.667 inches of runoff (V_{\min}) -

$$(V_{\min}) = (320 \text{ Ac.}) (0.667 \text{ inch}) (1 \text{ ft./12 in.})$$

$$= 17.78 \text{ AF}$$

Since (V_R) is more than (V_{\min}), 28.50 AF is correct for permanent wet pool volume (V_B) in this case.

Therefore, the wet detention system design pool volume
 $= (Q) 26.67 \text{ AF} + (V_B) 28.50 \text{ AF} = 55.17 \text{ AF}.$

3. Calculate the average minimum pond area (A_s) -

Based on treatment volume below control elevation of "v"-notch weir, ½ inch runoff and 10 in. maximum head or based on design pool volume at maximum depth -

1) Based on 10 in. maximum head on the "v"-notch:

$$(V_w) = (320 \text{ Ac.}) (0.50 \text{ inch}) (1 \text{ ft./12 in.})$$

$$= 13.33 \text{ AF}$$

$$(A_s) = (13.33 \text{ AF} / 0.833 \text{ ft.}) = 16.00 \text{ Ac.}$$

2) Based on design pool volume $[(Q) + (V_B) = 55.17 \text{ AF}]$ at maximum depths:

$$55.17 \text{ AF} = [(0.35) (2 \text{ ft.}) (A_s)] + [(0.65) (8 \text{ ft.}) (A_s)]$$

$$(A_s) = (55.17 \text{ AF}) / (5.9)$$

$$= 9.35 \text{ Ac.}$$

Check Max. head (H) = $(V_w) / (A_s)$,

$$(V_w) = 13.33 \text{ AF}; (A_s) = 9.35 \text{ Ac.}$$

$$(H) = (13.33/9.35) = 1.425 \text{ Ft.} = 17.1 \text{ in.} > 10 \text{ in.}$$

Therefore, the correct minimum pond area is 16.00 Ac.

COMMERCIAL EXAMPLE CALCULATION OF WET DETENTION DESIGN POOL VOLUME

Given: A shopping plaza project near Oneco, Florida; Project area = 16 Acres; Drainage area = 18 Acres; Composite Rational runoff coefficients: project site = 0.90; offsite = 0.45; drainage area = 0.85; Discharge occurs to Class III waters from a wet detention system.

- Required:**
1. Calculate the treatment volume; and
 2. Calculate the permanent wet pool volume to be retained below the control elevation to eight feet depth. It must be the greater of: a) the volume calculated to provide an average residence time of 14 days based on average total wet season rainfall of 31.04 inches; or, b) the volume produced by 0.667 inches of runoff from the contributing area; and
 3. Calculate the average minimum pond area.

1. Calculate the treatment volume (Q)

- a) For project site, as 1 inch of runoff (Q_p) -

$$\begin{aligned} (Q_p) &= (16 \text{ Ac.}) (1 \text{ inch}) (1 \text{ ft./12 in.}) \\ &= 1.33 \text{ Ac. -ft. (AF)} \end{aligned}$$

- b) For offsite, as runoff from first inch of rainfall (Q_o) -

$$\begin{aligned} (Q_o) &= (2 \text{ Ac.}) (1 \text{ inch}) (0.45) (1 \text{ ft./12 in.}) \\ &= 0.08 \text{ AF} \end{aligned}$$

Therefore, $(Q) = (Q_p) 1.33 \text{ AF} + (Q_o) 0.08 \text{ AF} = 1.41 \text{ AF}$

2. Calculate the permanent wet pool volume (V_B) -

a) Based on 14 day residence volume (V_R) -

$$(V_R) = (A) (C) (P) (R) (1 \text{ ft./12 in.})$$

Where, (A) = Project site + offsite = drainage area = 18 Ac.
 (C) = Composite Rational runoff coefficient = 0.85
 (P) = Historic average wet season rainfall rate for
 Arcadia, Bradenton, Brooksville, Lakeland and
 Ocala gauging stations = (31.04 in./122 days)
 (R) = Residence time = 14 days
 $(V_R) = (18) (0.85) (31.04/122) (14) (1/12)$
 $= 4.54 \text{ AF}$

NOTE: Refer to Figure 2 for graphic solution of 14 day residence volumes for various project types and sizes.

b) As 0.667 inches of runoff (V_{min}) -

$$(V_{min}) = (18 \text{ Ac.}) (0.667 \text{ inch}) (1 \text{ ft./12 in.})$$

$$= 1.00 \text{ AF}$$

Since (V_R) is more than (V_{min}), 4.54 AF is correct for permanent wet pool volume (V_B) in this case.

Therefore, the wet detention system design pool volume
 $= (Q) 1.41 \text{ AF} + (V_B) 4.54 \text{ AF} = 5.95 \text{ AF}.$

3. Calculate the average minimum pond area (A_s) -

Based on treatment volume below control elevation of "v"-notch weir, 1/2 inch runoff and 10 in. maximum head or based on design pool volume at maximum depth -

1) Based on 10 in. maximum head on the "v"-notch:

$$(V_W) = (18 \text{ Ac.}) (0.50 \text{ inch}) (1 \text{ ft./12 in.})$$

$$= 0.75 \text{ AF}$$

$$(A_s) = (0.75 \text{ AF} / 0.833 \text{ ft.}) = 0.90 \text{ Ac.}$$

- 2) Based on design pool volume $[(Q) + (V_B) = 5.95 \text{ AF}]$ at maximum depths (i.e., 35% @ 2' and 65% @ 8' depth):

$$5.95 \text{ AF} = [(0.35) (2 \text{ ft.}) (A_s)] + [(0.65) (8 \text{ ft.}) (A_s)]$$

$$\begin{aligned} (A_s) &= (5.95 \text{ AF}) / (5.9) \\ &= 1.01 \text{ Ac.} \end{aligned}$$

Check Max. head $(H) = (V_w) / (A_s)$,

$$(V_w) = 0.75 \text{ AF}; (A_s) = 1.01 \text{ Ac.}$$

$$(H) = (0.75/1.01) = 0.743 \text{ Ft.} = 8.9 \text{ in.} < 10 \text{ in.}$$

Therefore, the correct minimum pond area is 1.01 Ac.

REFERENCES:

1. "Permit Information Manual, Management and Storage of Surface Waters," March 1988 (Revised), SWFWMD, Brooksville, Florida.
2. "The Florida Development Manual: A Guide to Sound Land and Water Management," June 1988, FDER.
3. "Design of Urban Runoff Quality Controls," Proceedings of an Engineering Foundation Conference held in July 1988, American Society of Civil Engineers, 1989.
4. "Wet Detention Systems," A paper by Peter J. Singhofen, David W. Hamstra and Martin W. Pawlitkowski; 1990 Stormwater Management: A Designer's Course, the Florida Engineering Society, February 1990.
5. "Management and Storage of Surface Waters, Permit Information Manual, Volume IV," June 1987 (Revised), SWFWMD, West Palm Beach, Florida.
6. Clarification Memo No. CM/SWP-51, "Wet Detention Systems - Use of Gravity Bleeddown Orifices" (SWFWMD).

ATTACHMENTS:

- Figure 1. Discharge Structure End View and Discharge Structure Instream View.
- Figure 2 14-Day Residence Volume in Acre-Feet Per Acre of Contributing Area -
DISTRICT-WIDE.
- Figure 3 Discharge and Central Angle for a "V"-Notch Weir.
- Table A-1 Wet Detention Treatment, Conservation Design Pool Below SHWL Without
Discharge.

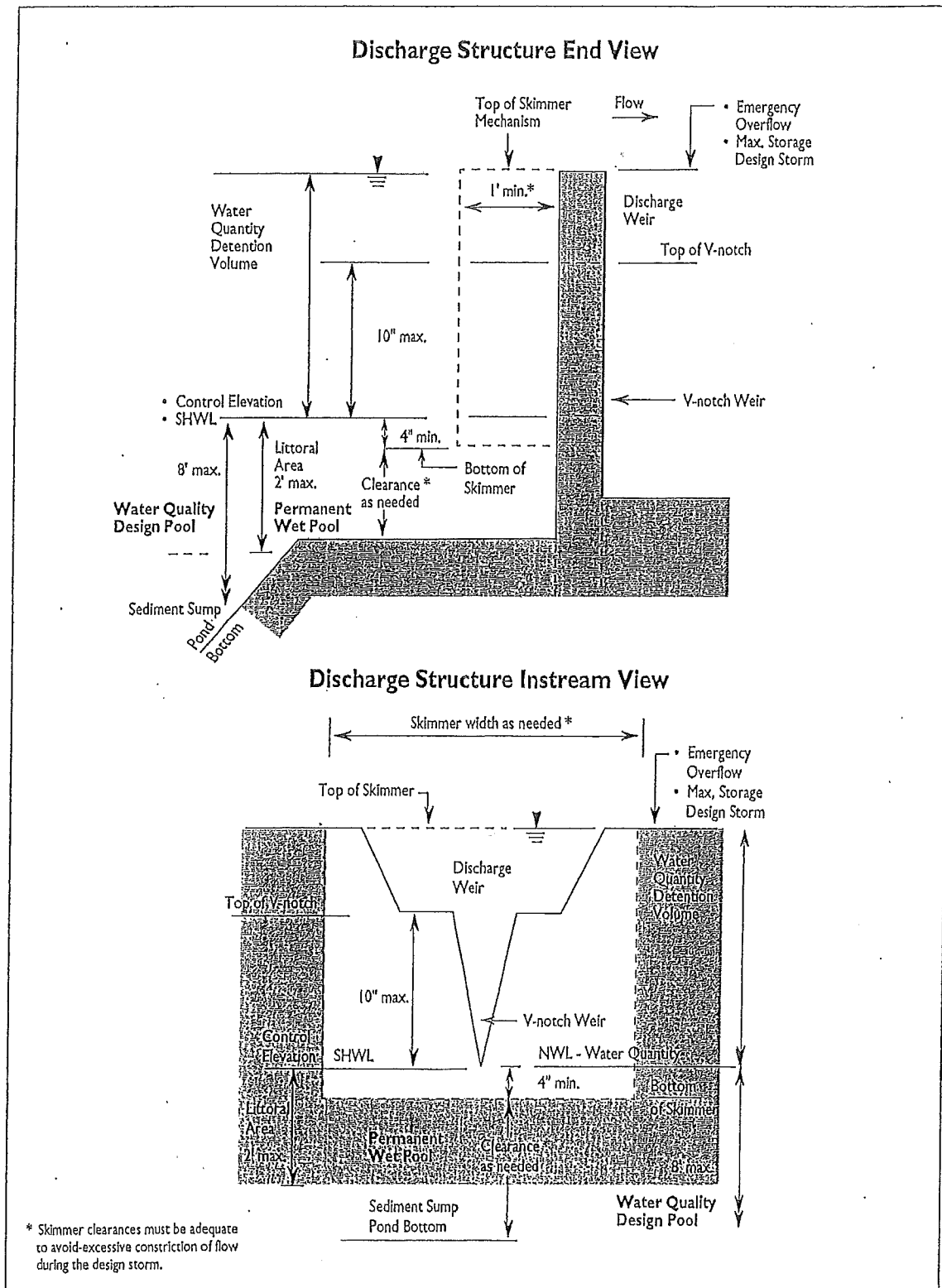


Figure 1

Table A-1

Wet Detention Treatment

CONSERVATION DESIGN POOL BELOW SHWL WITHOUT DISCHARGE

MANMADE WET DETENTION DESIGN AND PERFORMANCE STANDARDS	
Treatment Volume/Depth	1" runoff from on-site; runoff from first 1" of rainfall from offsite
Draw Down Time	Not required for treatment volume
Permanent Design Pool Volume	Rainy season 14 day residence volume plus treatment volume; minimum 1.667 inch runoff
Other Criteria for System Design	<ul style="list-style-type: none"> • 35% littoral zone @ control elevation; concentrated at outfall. • V-notch weir sized to discharge ½ inch runoff in 24 hours, 10" maximum flux. above SHWL/control elevation. • Littoral zone 2' maximum depth below control elevation. • Design pool, 8' maximum depth; 34% minimum pond bottom below SHWL. • Sediment sump and skimmer usually required. • Mulching or planting required if soils are unsuitable. • Side slopes 4H:1V unless safety fenced. • Inflow/outflow points must maximize circulation. • Control elevation not lower than SHWL and tailwater, nor higher than 2' above SHWL.

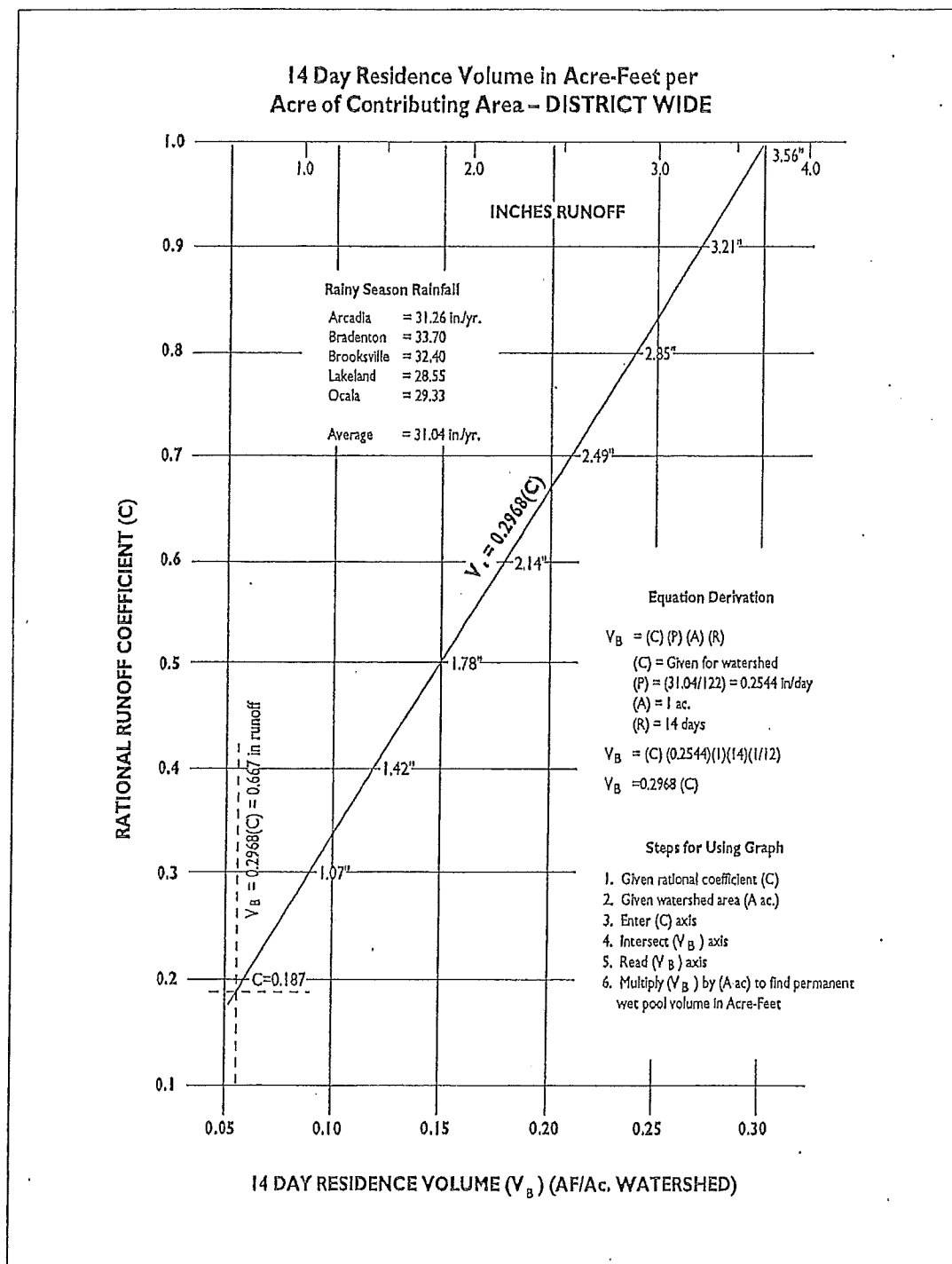
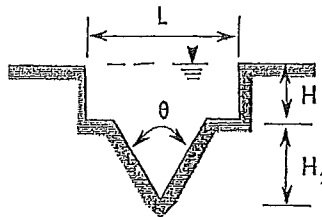


Figure 2

Discharge and Central Angle for a "V" - Notch Weir



The total flow over a rectangular sharp crested weir with a "V" - notch step discharge fluctuation device is approximated by the equation:

$$Q_D = 3.13 (L)(H_1)^{1.5} + 4.8 [(H_2)^2 \tan \theta/2] (H_1 + H_2/3)^{0.5}$$

Refer to Reference 1, pp. C-48 through C-50; and Reference 5, pp. C-IV-26 through C-IV-28.

Required V-Notch Size, θ

NOTE: V-Notch Size Required to Bleed-Down 0.5 Inch of Detention Volume in 24 Hours

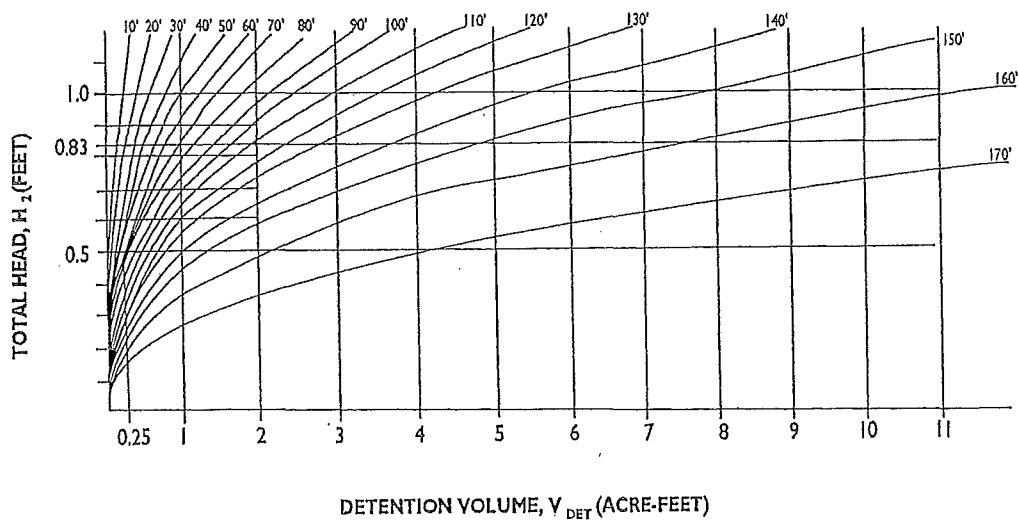


Figure 3

APPENDIX C

Figures Relating to Water Quality and Quantity Provisions

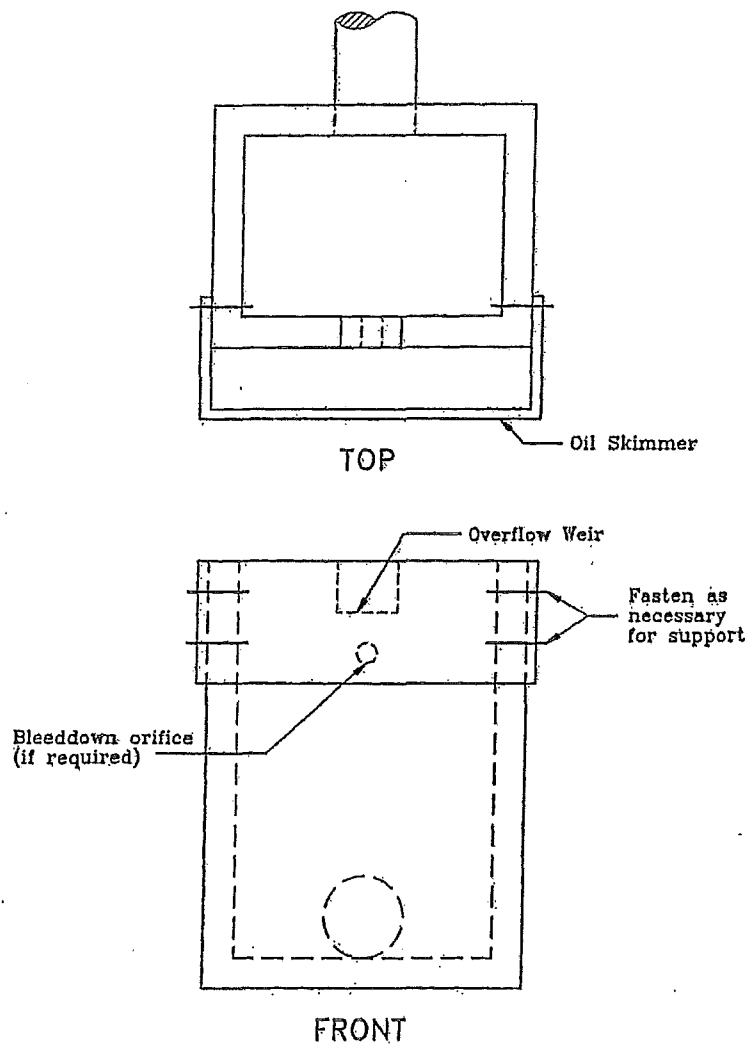


Figure 1. Oil Skimmer Detail for a Typical Outfall Structure (N.T.S.)

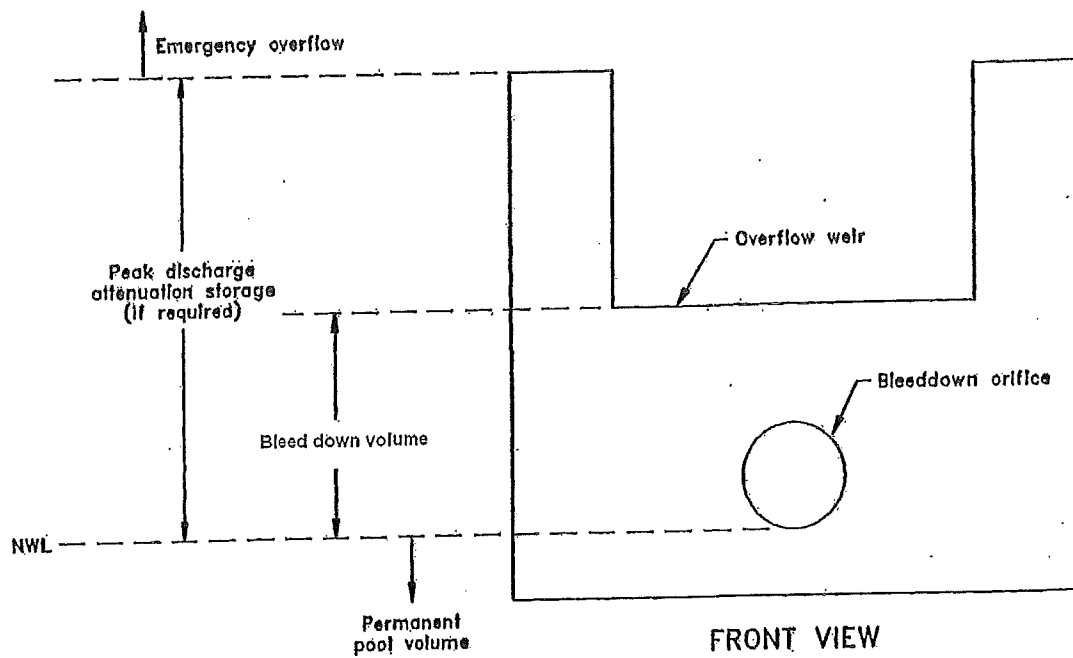


Figure 2. Typical Wet Detention Outfall Structure (N.T.S.)

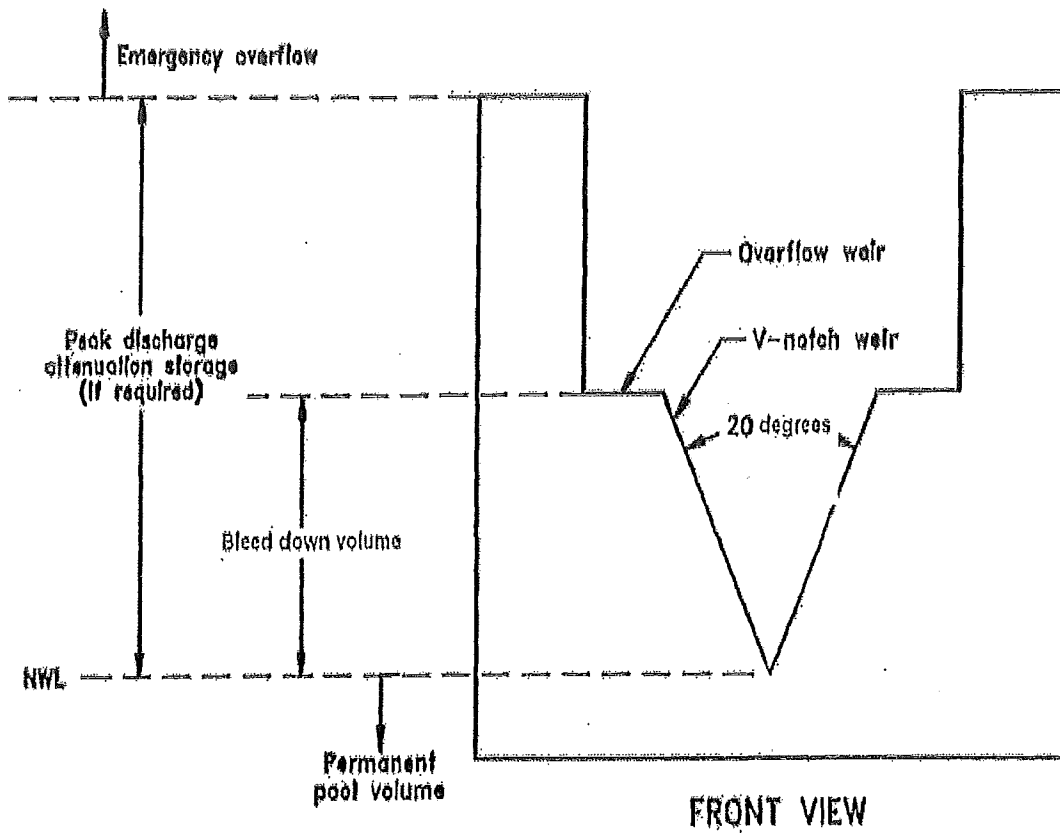


Figure 3. Typical Wet Detention Outfall Structure with "V"-notch Weir (N.T.S.)

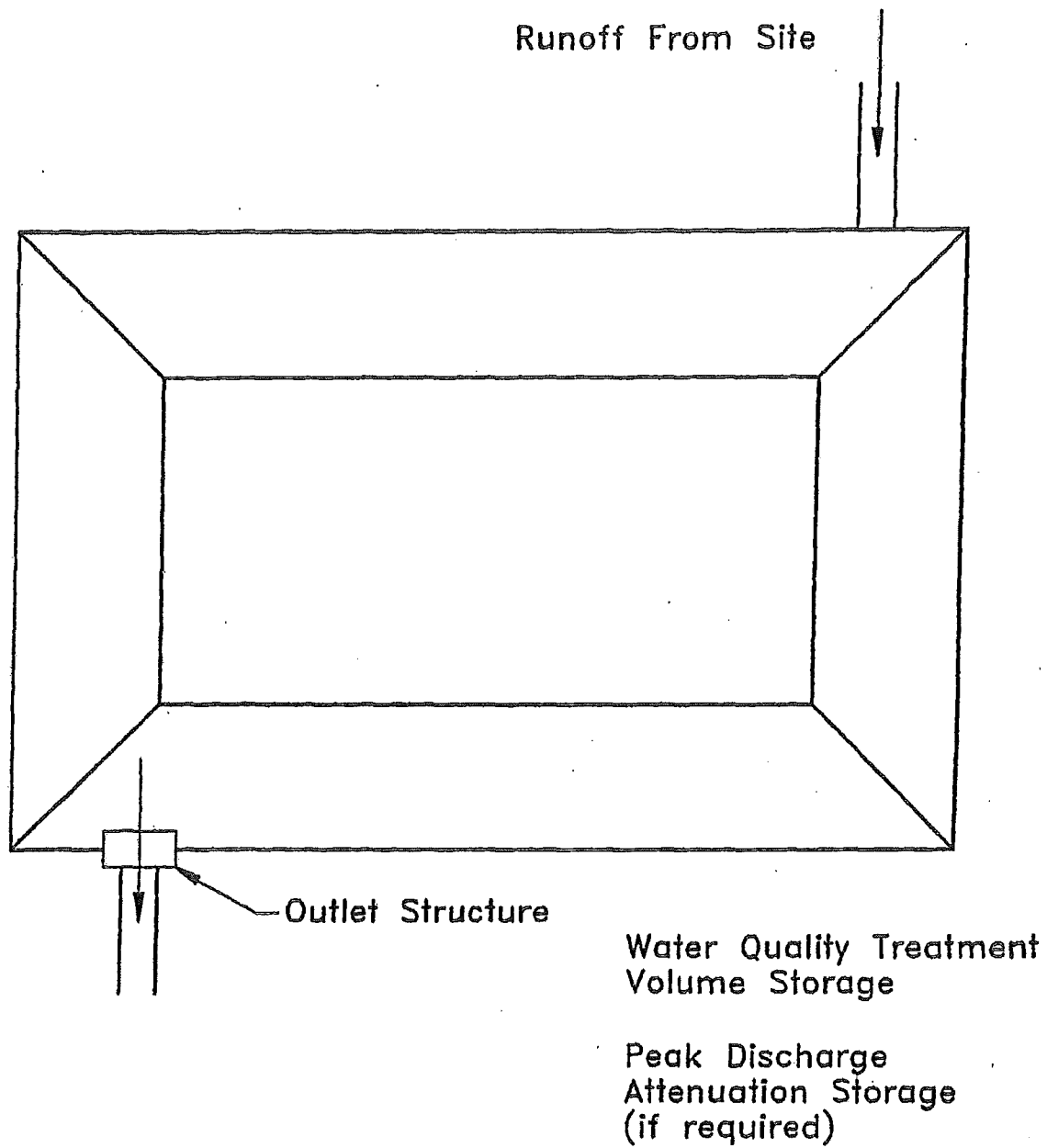


Figure 4. On-line treatment system.

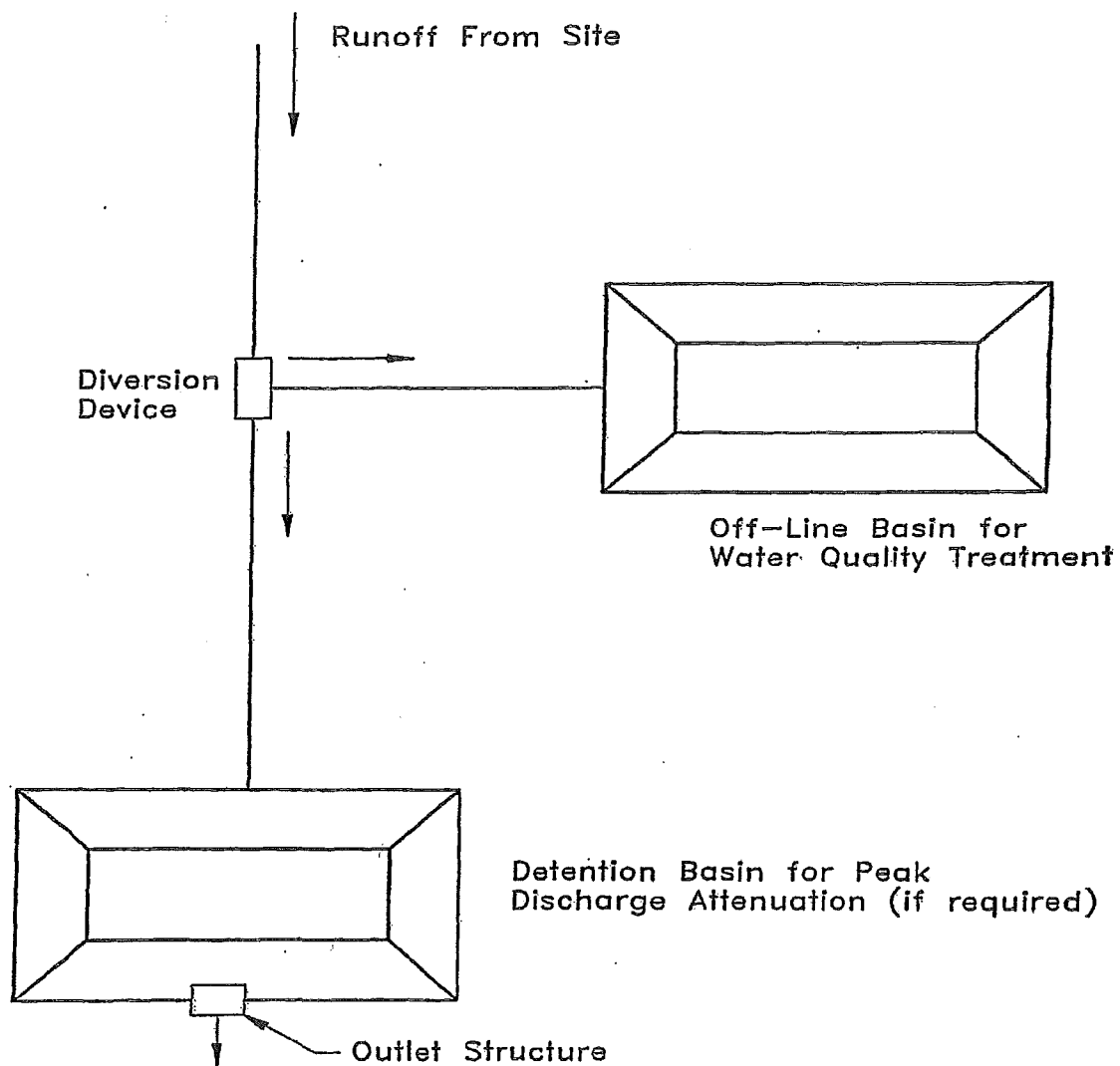


Figure 5. Off-line treatment system.

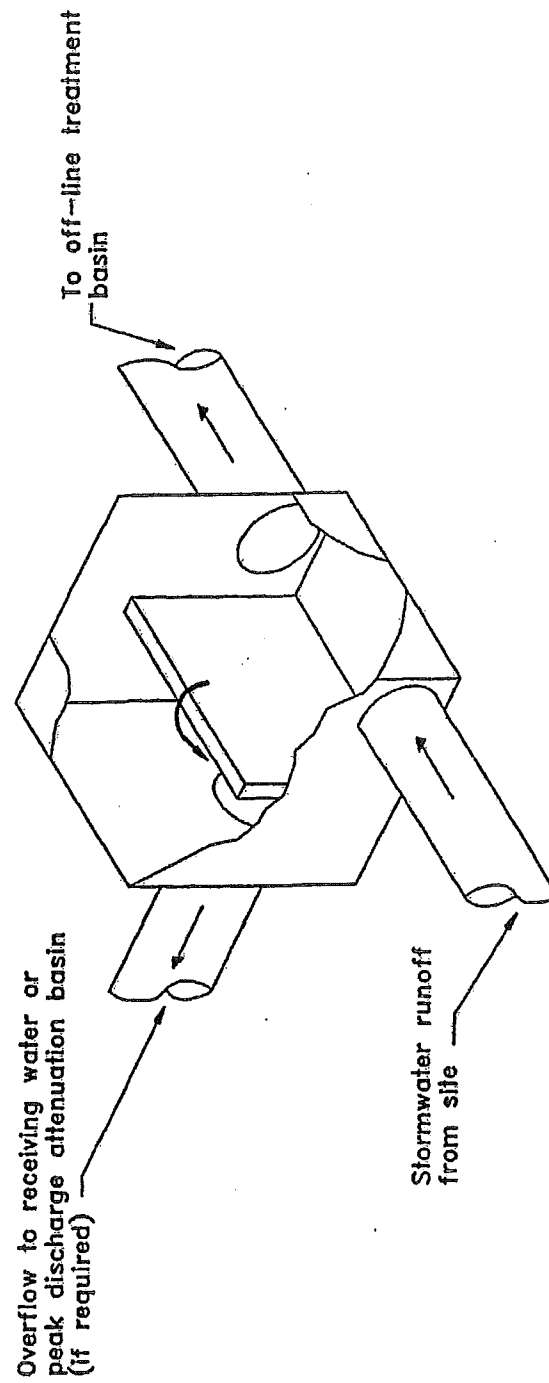


Figure 6. Diversion box (N.T.S.).

Generalized geologic section in Sensitive Karst Area with limestone at and near land surface

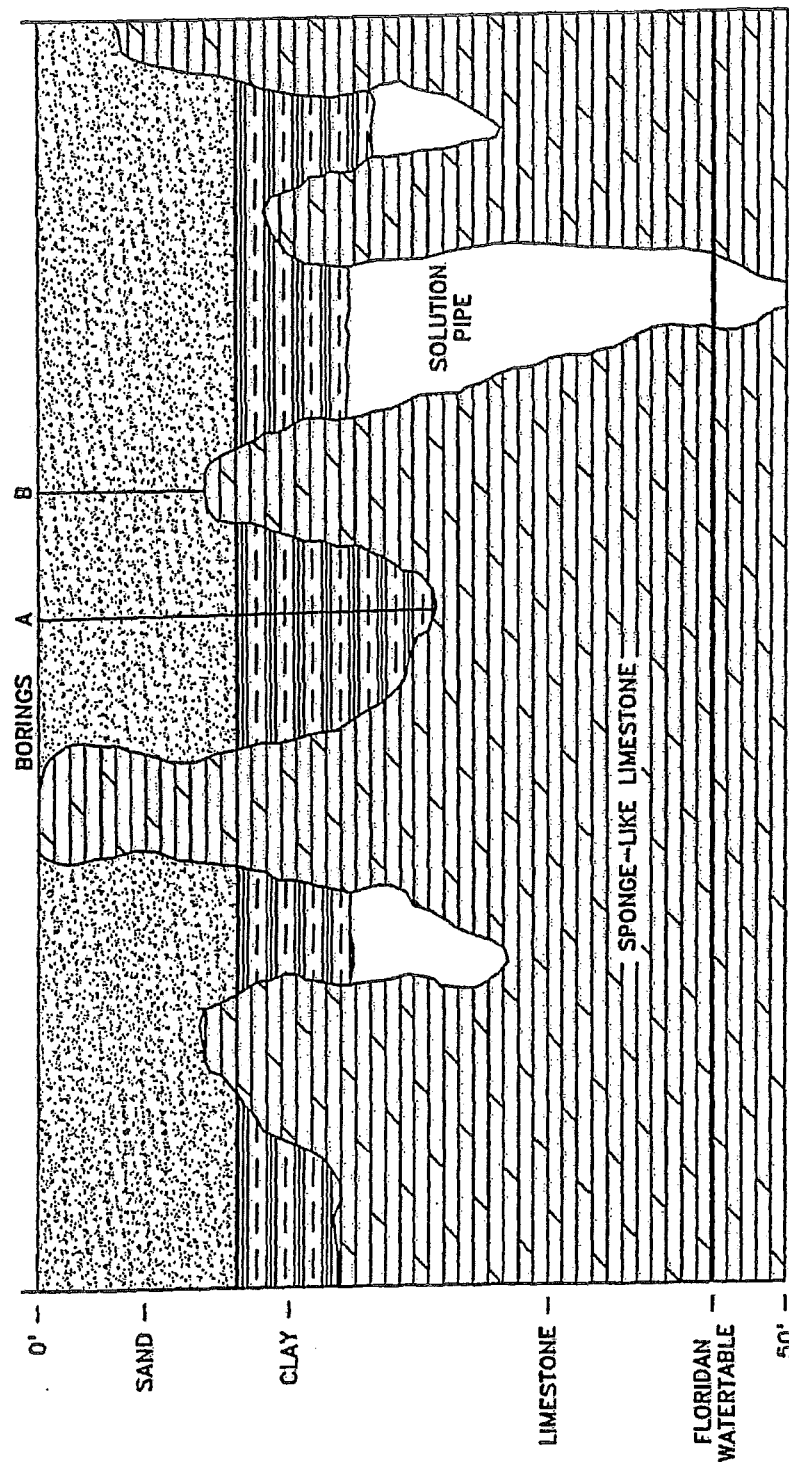


Figure 7. Generalized geologic section in Sensitive Karst Area with limestone at and near land surface

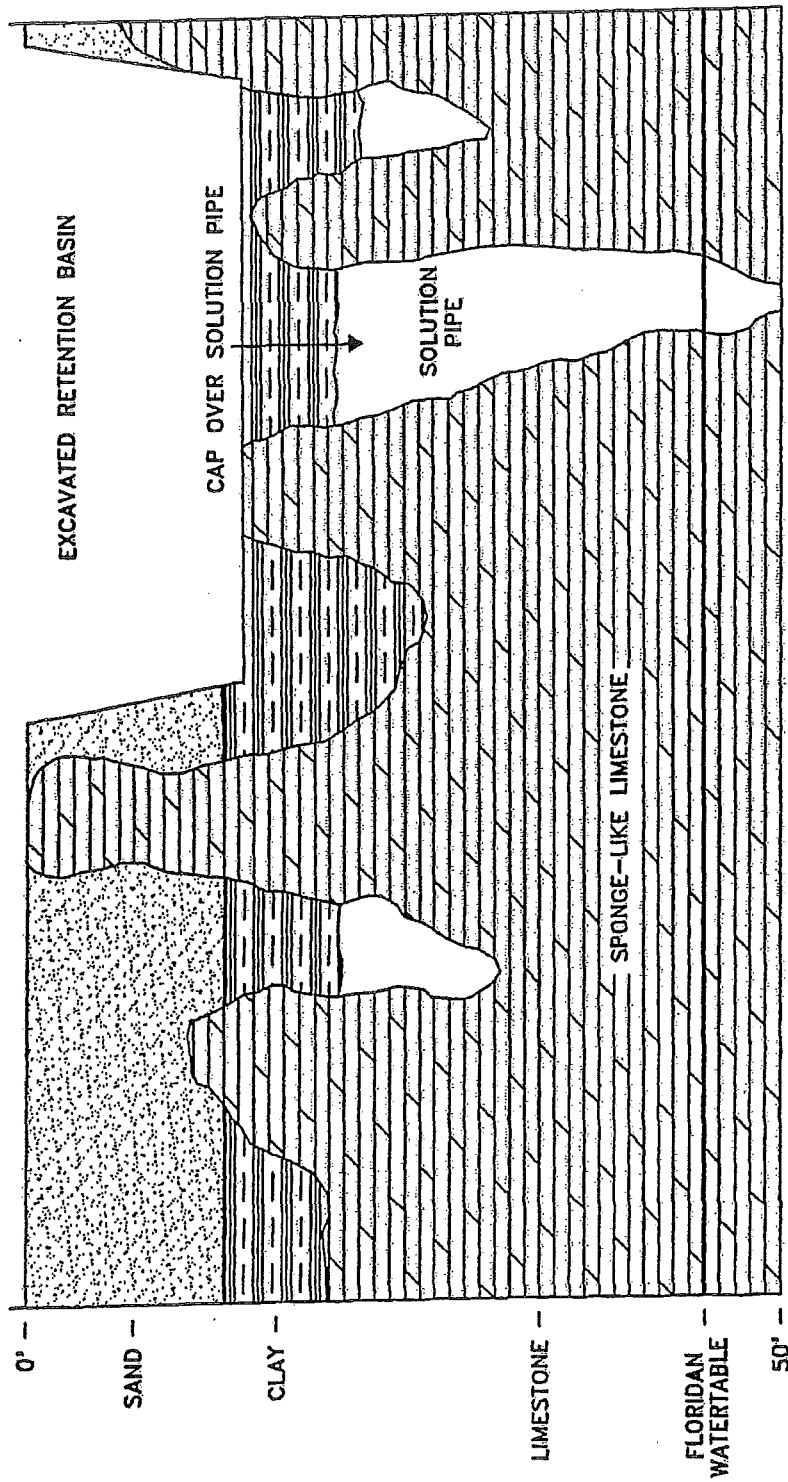


Figure 8. Retention basin added to Figure 7.

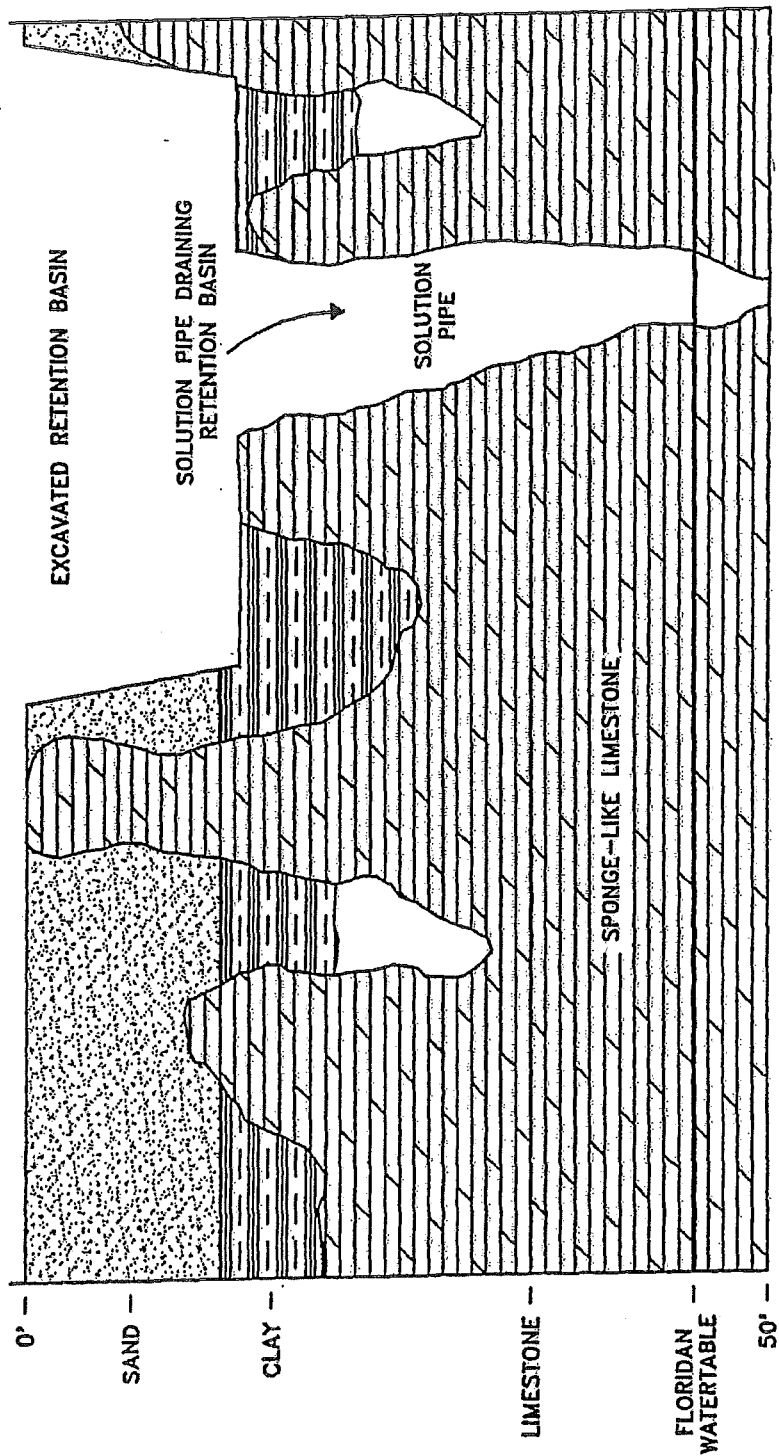


Figure 9. Retention pond with solution pipe

APPENDIX D

REFERENCES AND DESIGN AIDS TO ASSIST USERS IN DESIGNING BETTER STORMWATER TREATMENT SYSTEMS

The following references are provided for those who wish to obtain additional information about the effective design, construction, operation, and maintenance of stormwater treatment systems.

The Natural Resources Conservation Service (NRCS) **National Engineering Handbook** (NEH) has been revised over the past several years, and is still undergoing periodic revisions to its numerous Parts and Chapters. The entire NEH is currently available on line at:
<http://www.nrcs.usda.gov/technical/engineering/neh.html>

The "hydrology" section of the NEH is now available under *Part 630 – Hydrology*, which consists of twenty-two (22) Chapters. These 22 Chapters are available on line at:
<http://directives.sc.egov.usda.gov/viewerFS.aspx?id=2572>
As a point of information, **Chapter 16 – Hydrographs** (dated March, 2007) is available via this same URL.

The Florida Department of Transportation (FDOT) **Drainage Manual** has also been revised over the past several years, and is still undergoing periodic revisions to its various "Handbooks" contained within the Drainage Manual. These updated publications are currently available on line at:
<http://www.dot.state.fl.us/rddesign/dr/Manualsandhandbooks.shtm>

The "Rational Method" (for generating peak flow rates only) and the "Modified Rational Method" (for generating hydrographs) can be found in sections 2.2.3 and 2.2.4 of the February 2012 **Drainage Handbook – Hydrology**, available at the above referenced URL.

The Laws and Rules of regulated professions in Florida can be accessed at the following web addresses:

Florida Statutes:

http://www.leg.state.fl.us/STATUTES/index.cfm?App_mode=Display_Index&Title_Request=XXXII#TitleXXXII

Rules (Florida Administrative Code):

<https://www.flrules.org/Default.asp>

Soil Surveys and Official Soil Series Descriptions are available through the NRCS Web Soil Survey which is accessible at:

<http://websoilsurvey.nrcs.usda.gov/app/HomePage.htm>
<http://www.dep.state.fl.us/water/nonpoint/docs/nonpoint/May04StSweepGuidance.pdf>

1/22/2013 Draft

Sod Farm Operations:

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_FloridaSod2008.pdf

Specialty Fruit & Nut Crop Operations: includes blueberries, nuts, peaches, plums, nectarines, avocados, lychees, mamey, papayas, grapes, blackberries, raspberries, or other similar fruits and nuts.

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_FloridaSpecialtyFruitNut2011.pdf

Silvicultural Operations:

http://www.floridaforestservice.com/publications/silvicultural_bmp_manual2011.pdf

Aquacultural Operations:

http://www.floridaaquaculture.com/publications/P-01499-booklet-07_BMP_RULE.pdf

APPENDIX E

AGRICULTURAL BEST MANAGEMENT PRACTICES MANUALS

An important component of a water resource sustainable agricultural operation is the utilization of site specific nutrient, pest, drainage and irrigation best management practices (BMPs). The grower may contact the United States Department of Agriculture Natural Resources Conservation Service (USDA-NRCS) to obtain a federally prescribed Resource Management System (RMS) plan of site specific BMPs as part of the District's agricultural exemption confirmation process. A listing of local USDA-NRCS offices may be found at <http://www.fl.nrcs.usda.gov/contact/index.html>.

As an option, the Florida Department of Agriculture and Consumer Services (FDACS) Office of Agricultural Water Policy also has rule adopted statewide BMP rules and manuals for the major commodity crops. Signing their Notice of Intent (NOI) and implementing the prescribed BMPs provides a presumption of compliance with statewide water quality discharge standards. These crop-specific adopted BMP manuals are found on the FDACS website links listed below and can be found in Chapters 5M-2 through 5M-14, F.A.C. To review the FDACS rules adopting BMPs, visit the Florida Department of State at: <https://www.flrules.org/Default.asp>

Citrus Groves:

- (a) Citrus on Well Drained Ridge Soils:

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_RidgeCitrus2002.pdf

- (b) Citrus on Poorly Drained Flatwood Soils & Within the Peace River Watershed:

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_PeaceRiverCitrus2004.pdf

Containerized Nurseries:

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_FloridaContainerNurseries2007.pdf

Equine Operations:

<http://www.floridaagwaterpolicy.com/BestManagementPractices.html>

(then choose Florida Equine Operations BMP Manual pdf directly)

Cow/Calf Operations:

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_FloridaCowCalf2008.pdf

Row Crop Operations: includes vegetables and other agronomic crops like potatoes, corn, soybeans, peanuts, peppers, sugarcane, cotton, tomatoes, cucumbers, strawberries, squash, and watermelons.

http://www.floridaagwaterpolicy.com/PDF/Bmps/Bmp_VeggieAgroCrops2005.pdf

Rule Development Workshop Proposed ERP Applicant's Handbook II

to be adopted in 40D-4.091 and 62-
330.010, F.A.C., as part of SWFWMD
Implementation of Statewide ERP Rules
(SWERP)

January 22, 2013
9 AM

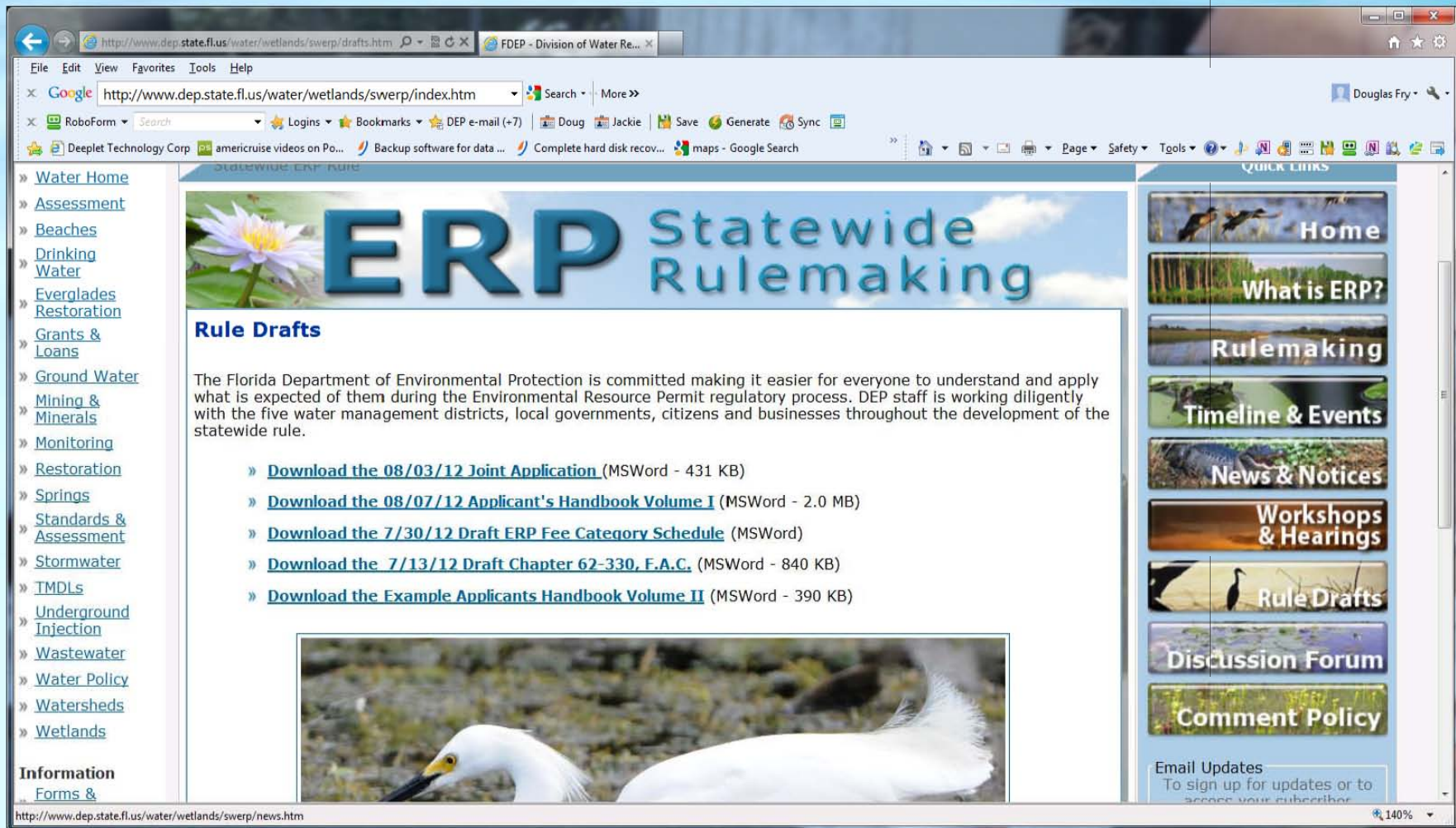
Workshop Agenda

- Call to Order
- Introductions and Opening Remarks
- Presentation
 - Statewide Environmental Resource Permitting (SWERP) Overview
 - SWFWMD Rule Changes – Tied to proposed SWERP
 - Proposed Applicant's Handbook II
- Public Comments and Questions
- Closing

SWERP Overview

- HB 7003 (373.4131, F.S.)
- Authorizes DEP, in conjunction with WMD's, to adopt a statewide ERP Rule
- Simplify and streamline ERP program
- Improve consistency statewide

Rulemaking Information



The screenshot shows a web browser window displaying the FDEP Statewide ERP Rulemaking website. The browser's address bar shows the URL <http://www.dep.state.fl.us/water/wetlands/swerp/drafts.htm>. The website features a navigation menu on the left with links to various water-related topics, a main content area with a large banner for 'ERP Statewide Rulemaking' and a list of rule drafts for download, and a 'QUICK LINKS' sidebar on the right with buttons for Home, What is ERP?, Rulemaking, Timeline & Events, News & Notices, Workshops & Hearings, Rule Drafts, Discussion Forum, and Comment Policy. The main content area also includes a paragraph about the FDEP's commitment to making the regulatory process easier and a list of downloadable documents with their file sizes. A photograph of a white egret is visible at the bottom of the main content area.

Water Home
» [Assessment](#)
» [Beaches](#)
» [Drinking Water](#)
» [Everglades Restoration](#)
» [Grants & Loans](#)
» [Ground Water](#)
» [Mining & Minerals](#)
» [Monitoring](#)
» [Restoration](#)
» [Springs](#)
» [Standards & Assessment](#)
» [Stormwater](#)
» [TMDLs](#)
» [Underground Injection](#)
» [Wastewater](#)
» [Water Policy](#)
» [Watersheds](#)
» [Wetlands](#)

Information
» [Forms &](#)

ERP Statewide Rulemaking

Rule Drafts

The Florida Department of Environmental Protection is committed making it easier for everyone to understand and apply what is expected of them during the Environmental Resource Permit regulatory process. DEP staff is working diligently with the five water management districts, local governments, citizens and businesses throughout the development of the statewide rule.

- » [Download the 08/03/12 Joint Application](#) (MSWord - 431 KB)
- » [Download the 08/07/12 Applicant's Handbook Volume I](#) (MSWord - 2.0 MB)
- » [Download the 7/30/12 Draft ERP Fee Category Schedule](#) (MSWord)
- » [Download the 7/13/12 Draft Chapter 62-330, F.A.C.](#) (MSWord - 840 KB)
- » [Download the Example Applicants Handbook Volume II](#) (MSWord - 390 KB)

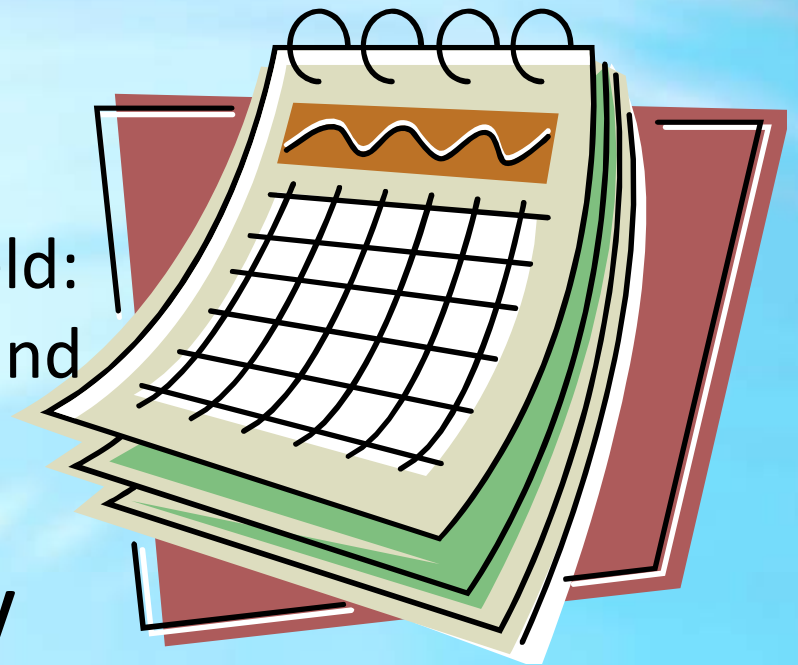
QUICK LINKS

- [Home](#)
- [What is ERP?](#)
- [Rulemaking](#)
- [Timeline & Events](#)
- [News & Notices](#)
- [Workshops & Hearings](#)
- [Rule Drafts](#)
- [Discussion Forum](#)
- [Comment Policy](#)

Email Updates
To sign up for updates or to access your subscriber

Public Rulemaking Workshops

- Workshops broadcast via webinar statewide
- Five completed Workshops Held: July 26, August 7, August 16, and September 20, December 14
- Next Workshop: **Late February**



Rulemaking Schedule

Date

Event

Late Feb 2013

FDEP Public Workshop

Spring 2013

FDEP & SWFWMD Publish NPR

Summer 2013

Rule Effective Date

Summary of SWERP Changes

What's Changing

- Permit types and thresholds
- Conceptual approvals
- Fee categories
- Statewide forms
- New and amended exemptions and NGPs

What's NOT Changing

- Environmental criteria
- Quantity/quality design criteria
- Mitigation banking
- UMAM
- Ag exemptions/permitting
- SSL's and Aquatic Preserves

SWFWMD Rule Amendments

- 40D-4.091 – amend to incorporate AH II by reference
- 40D-1.603 – adopt standard fee schedule established by DEP (not uniform fees)
- Existing 40D-4, 40 and 400 – will apply to grandfathered projects
- Chapter 40D-1 Procedures – amend as needed
 - ERP forms, noticing, processing procedures

Amendments to 40D-4.091, F.A.C.


40D-4.091 Publications, Forms and Agreements Incorporated by Reference.

The following documents are hereby incorporated by reference and are applicable to this chapter and Chapters 40D-40 and 40D-400, F.A.C.:

(1) – (6) No change.

(7) Southwest Florida Water Management District Environmental Resource Permitting Applicant's Handbook Volume II (effective date).
The document is available from the District's website at www.WaterMatters.org/permits/rules and from the District upon request. Applicant's Handbook Volume II applies only to applications, notices and petitions for formal or informal delineation that are processed under the statewide environmental resource permit rule adopted by the Department of Environmental Protection as Chapter 62-330, F.A.C.(effective date).

SWERP Applicant's Handbook

- Structured after NFWFMD Handbook format
- SWFWMD ERP Basis of Review  Applicant's Handbook
- Two Volumes – AH I and AH II
- AH I – applies statewide and will be the same for DEP and all WMD's – Administrative, Environmental, Sediment & Erosion Control
- AH II – Follows DEP specified outline and contains District-specific water quantity & quality criteria, special requirements

ERP AH II vs Basis of Review

- Environmental and Administrative Criteria - AH I
- FDOT Flexible Criteria (associated with HB 599) – Additional languages changes being considered
- Definitions – Minor Revisions and in both AH's
- Tailwater Considerations – Expanded Language
- Retrofits
- Rural Subdivision Criteria
- Wet- Detention – Conservation Design in Appendix
- Compensating Stormwater Treatment Considerations
- Sensitive Karst Areas – Design Considerations
- Dam Safety
- AGSWM Program Section
- Appendices

Public Comments and Questions?

Permit Thresholds and Types

- Revised permit thresholds
- Permit required if threshold is exceeded and activity doesn't qualify for an exemption
- Two types of permits – Generals (currently called NGP's) and Individual Permits
- No more General (current 40D-40 version), General for Minor Systems, Site Conditions Assessment Permits, or Short Form
- Modifications - Major or Minor

Exemptions and NGP's

- Existing exemptions mostly unchanged
- Some new exemptions – Related to roadway minor works and paving/re-paving
- Minor revisions to existing NGPs and repeal of two NGP's
- Several New NGP's

Post Permitting – Asbuilts and Re-Certifications

- Asbuilt certifications by registered professionals
- Permit drawings will function as asbuilt drawings
- Surveyed asbuilt drawings not submitted, unless needed to identify significant deviations
- Standard inspection cycles depending on the type of treatment system
- Permittee must maintain inspection records and submit only upon request by District staff or as required by special permit conditions